

# Introduction to Strong Motion Seismology

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SSA/EERI Tutorial

4/21/06

# Probabilistic Methods

# Deterministic Approach

- Select a small number of individual earthquake scenarios:  $M, R(\text{Location})$  pairs
- Compute the ground motion for each scenario (typically use ground motion with 50% or 16% chance of being exceeded if the selected scenario earthquake occurs)
- Select the largest ground motion from any of the scenarios

# Probabilistic Approach

- Source Characterization
  - Develop a comprehensive set of possible scenario earthquakes:  $M$ ,  $R(\text{location})$
  - Specify the rate at which each scenario earthquake  $(M,R)$  occurs
- Ground Motion Characterization
  - Develop a full range of possible ground motions for each earthquake scenario ( $\epsilon$ =number of std dev above or below the median)
  - Specify the probability of each ground motion for each scenario

# Probabilistic Approach (cont)

- Hazard Calculation
  - Rank scenarios (M,R,  $\epsilon$ ) in order of decreasing severity of shaking (Here, use  $S_a$ )
  - Result: Table of ranked scenarios with ground motions and rates
  - Sum up rates of scenarios with ground motion above a specified level (hazard curve)
- Select a ground motion for the design hazard level
  - Back off from worst case ground motion until either:
    - The ground motion is does not lead to excessive costs, or
    - The hazard level is not too small (e.g. not too rare) to ignore (e.g. the design hazard level)

# PSHA Calculation

- Standard form of hazard

$$v(Sa > z) = \sum_{i=1}^{nSource} N_i(M_{\min}) \int \int_{MR} f_{mi}(M) f_{Ri}(r, M) P(Sa > z | m, R) dR dM$$

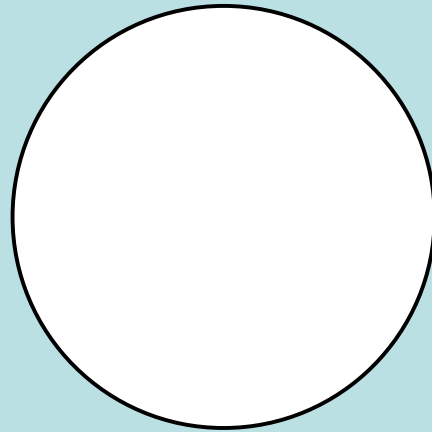
- Alternative form (explicit ground motion aleatory variability)

$$v(Sa > z) = \sum_{i=1}^{nSource} N_i(M_{\min}) \int \int \int_{MR\epsilon} f_{mi}(M) f_{Ri}(r, M) f_{\epsilon}(\epsilon) P(Sa > z | m, R, \epsilon) d\epsilon dR dM$$

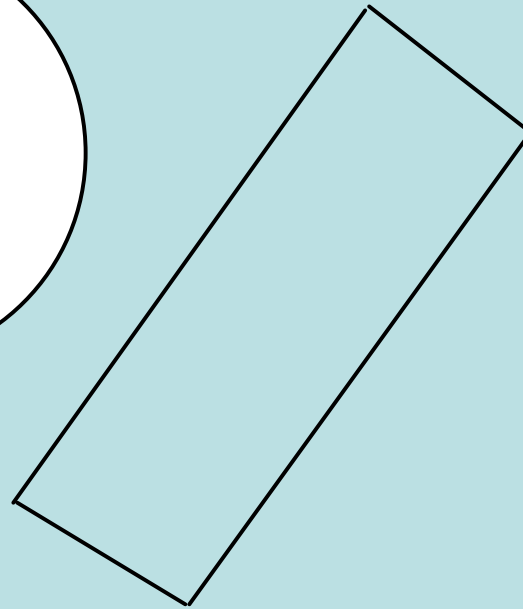
# Example Sources



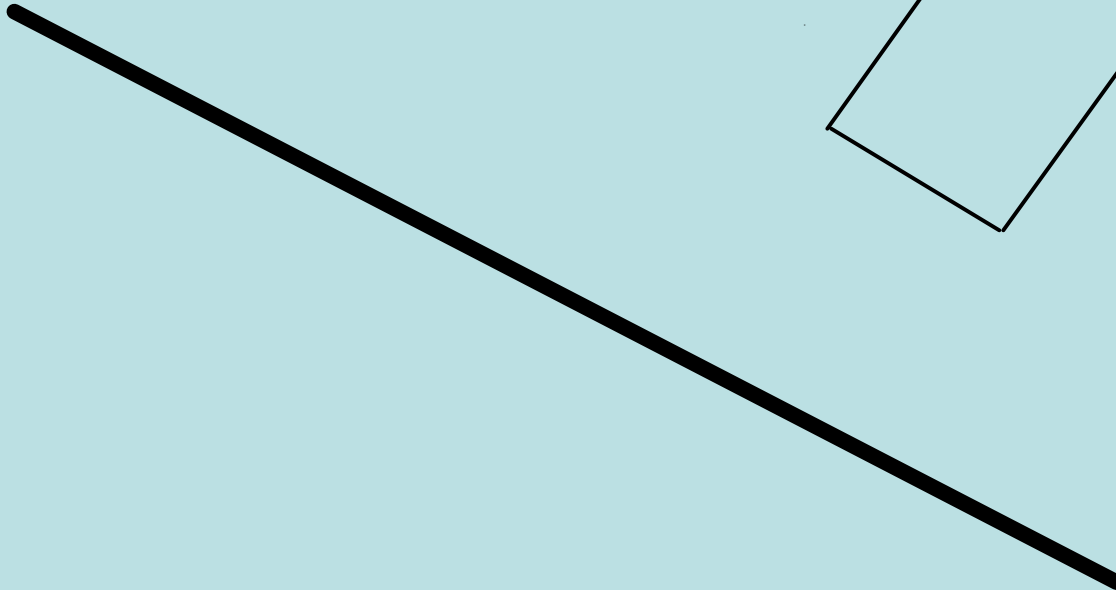
source 1



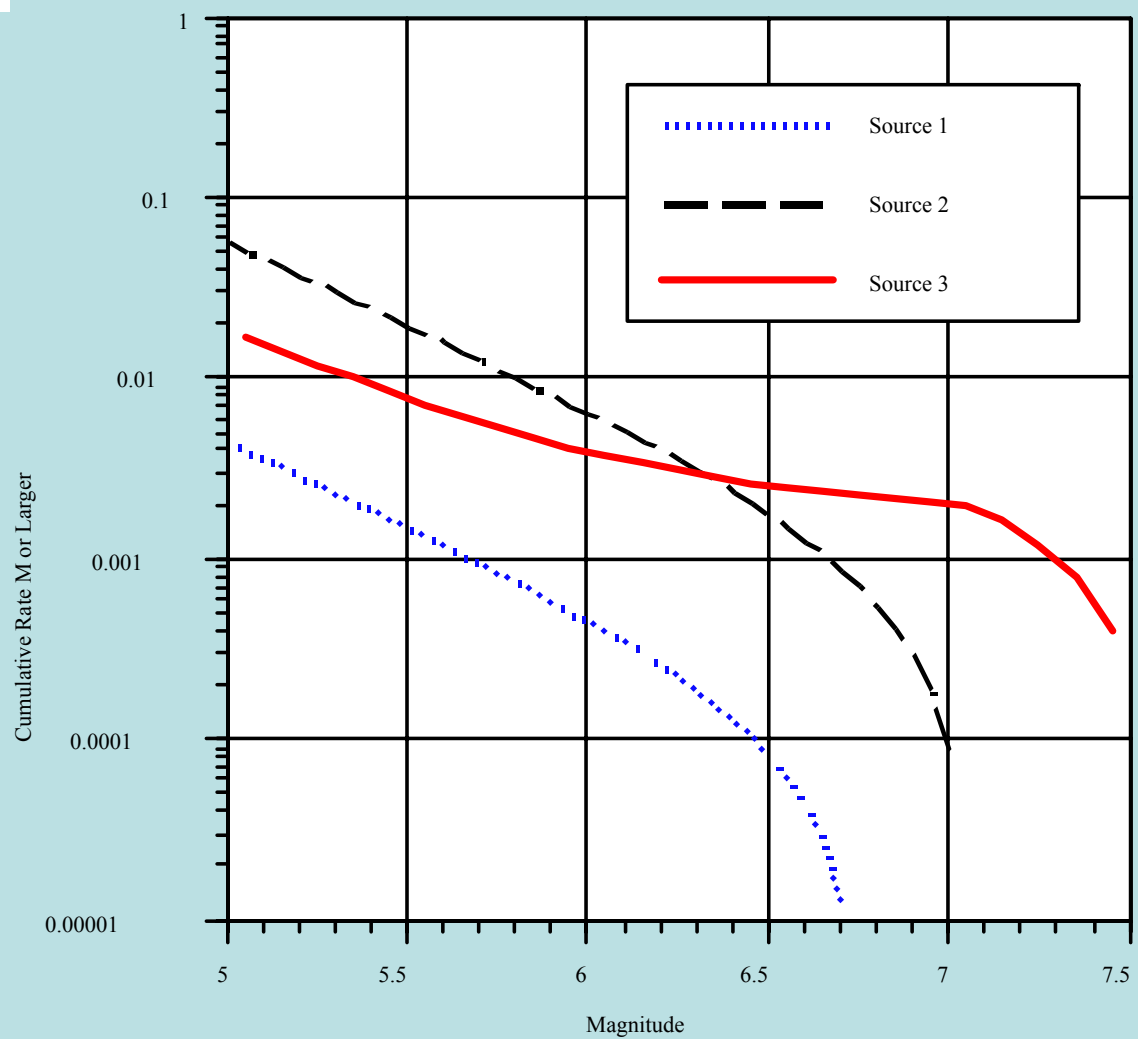
source 2



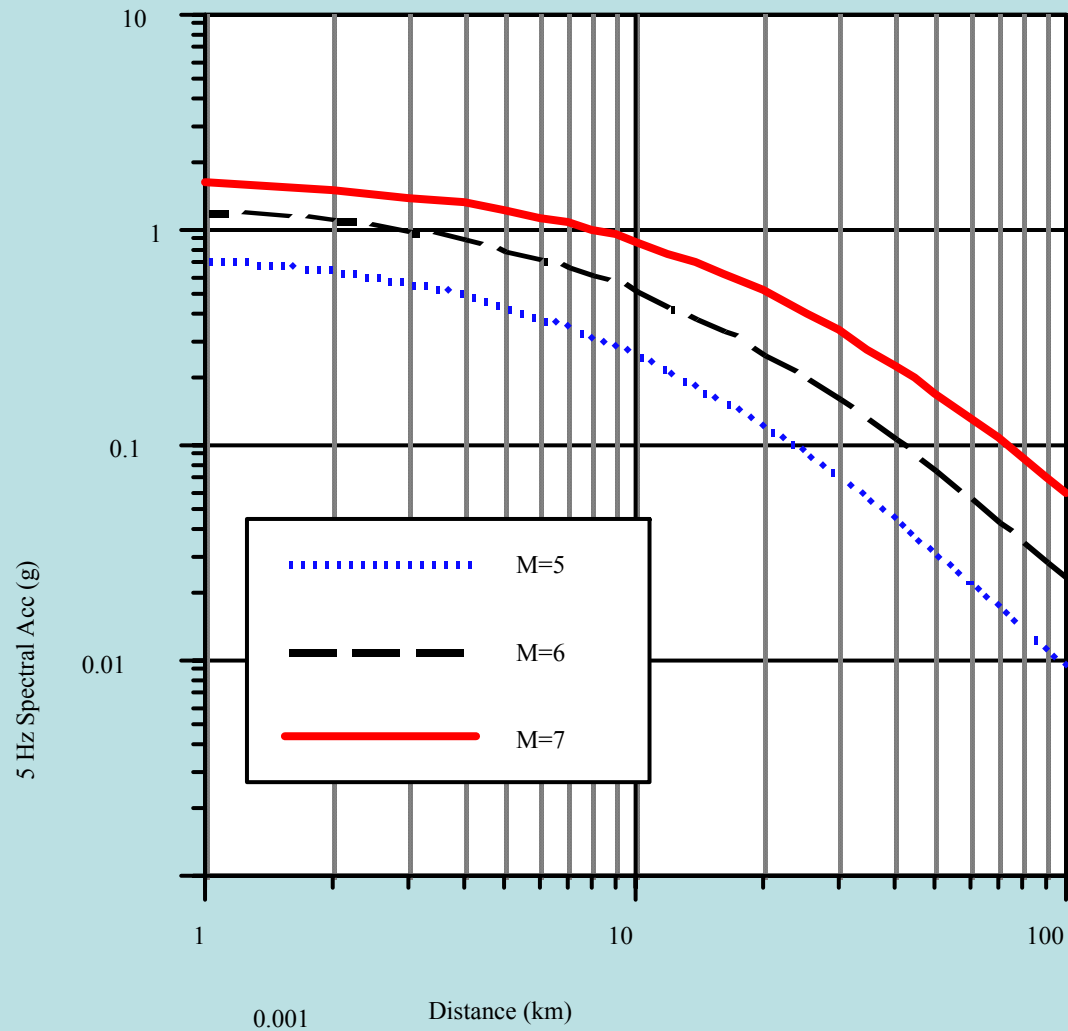
source 3



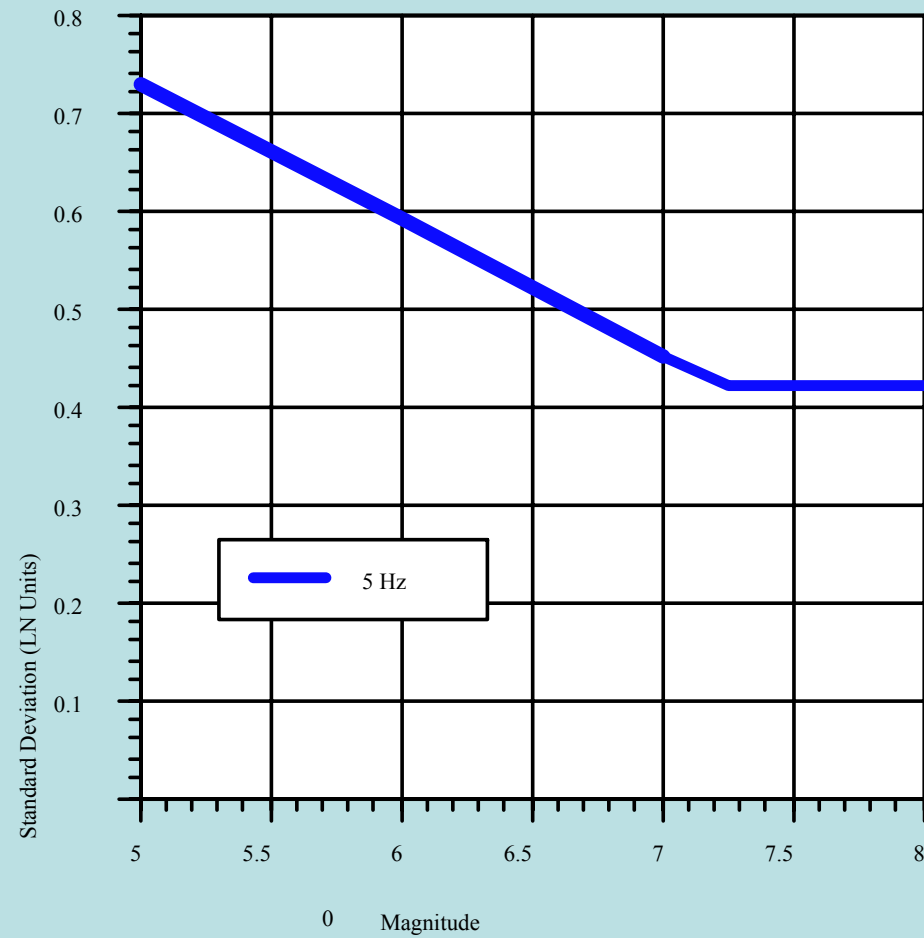
# Example Source Characterization



# Median Ground Motion



# Standard Deviation of Ground Motion



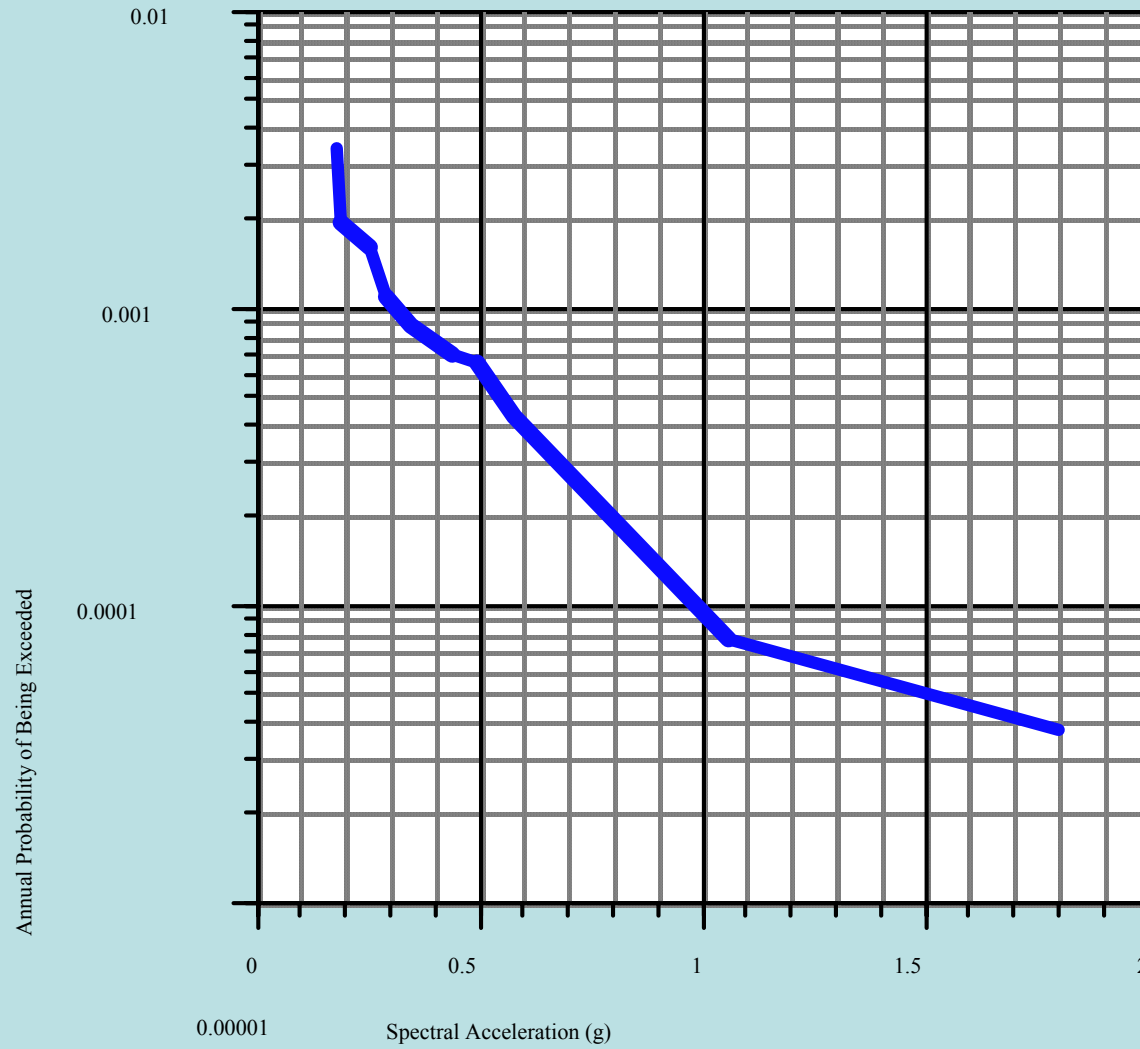
## Partial List of Scenarios

Source	Mag	R (km)	Rate of Sa	Median Sa	Std Dev	$\epsilon$	P(e)	Sa(g)	Rate
1	6.50	2	0.00022	1.38	0.53	0.5	0.175	1.80	0.000038
1	6.50	2	0.00022	1.38	0.53	-0.5	0.175	1.06	0.000038
1	5.00	2	0.00180	0.58	0.73	0.0	0.197	0.58	0.000355
1	5.00	10	0.00180	0.24	0.73	1.0	0.121	0.49	0.000218
2	5.50	40	0.02216	0.07	0.66	1.5	0.066	0.18	0.001453
2	6.00	40	0.00786	0.10	0.59	1.5	0.066	0.25	0.000516
2	6.50	40	0.00279	0.16	0.52	1.5	0.066	0.35	0.000183
3	7.25	60	0.00170	0.19	0.42	2.0	0.028	0.44	0.000047
3	7.25	60	0.00170	0.19	0.42	1.0	0.121	0.29	0.000206
3	7.25	60	0.00170	0.19	0.42	0.0	0.197	0.19	0.000336

## Rank Scenarios by Ground Motion

Source	Mag	R (km)	$\varepsilon$	Sa(g)	Rate	Hazard
1	6.50	2	0.5	1.80	0.000038	0.000038
1	6.50	2	-0.5	1.06	0.000038	0.000076
1	5.00	10	0.0	0.58	0.000355	0.000432
3	7.25	60	1.0	0.49	0.000218	0.000649
2	6.50	40	1.5	0.44	0.000047	0.000697
3	7.25	60	1.5	0.35	0.000183	0.000880
1	5.00	2	1.5	0.29	0.000206	0.001085
2	6.00	40	2.0	0.25	0.000516	0.001601
3	7.25	60	1.0	0.19	0.000336	0.001937
2	5.50	40	0.0	0.18	0.001453	0.003390

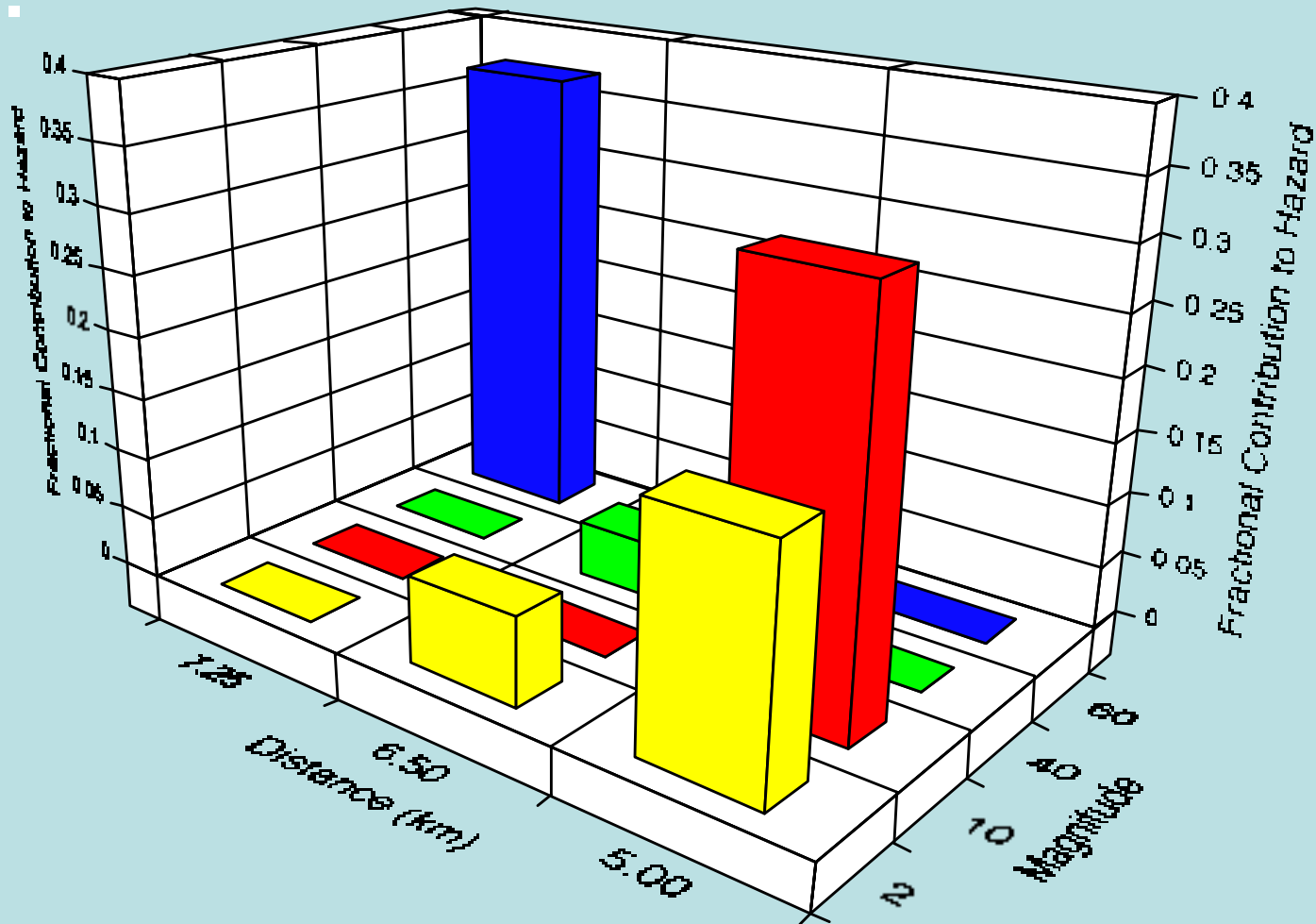
# Hazard Curve



# Deaggregation at $10^{-3}$ Hazard

Source	Mag	R (km)	$\epsilon$	Sa(g)	Rate	Hazard	Deagg
1	6.50	2	0.5	1.80	0.000038	0.000038	0.035
1	6.50	2	-0.5	1.06	0.000038	0.000076	0.035
1	5.00	10	0.0	0.58	0.000355	0.000432	0.327
3	7.25	60	1.0	0.49	0.000218	0.000649	0.201
2	6.50	40	1.5	0.44	0.000047	0.000697	0.044
3	7.25	60	1.5	0.35	0.000183	0.000880	0.169
1	5.00	2	1.5	0.29	0.000206	0.001085	0.190
2	6.00	40	2.0	0.25	0.000516	0.001601	
3	7.25	60	1.0	0.19	0.000336	0.001937	
2	5.50	40	0.0	0.18	0.001453	0.003390	

# Group Similar Scenarios for Deaggregation Plots



# Aleatory Variability and Epistemic Uncertainty

- Scientific Uncertainty (epistemic)
  - Due to lack of information
  - Incorporated in PSHA using logic trees (leads to alternative hazard curves)
  - Impacts the mean hazard
- Random Variability (aleatory)
  - Randomness in  $M$ , location, ground motion ( $\varepsilon$ )
  - Incorporated in hazard calculation directly

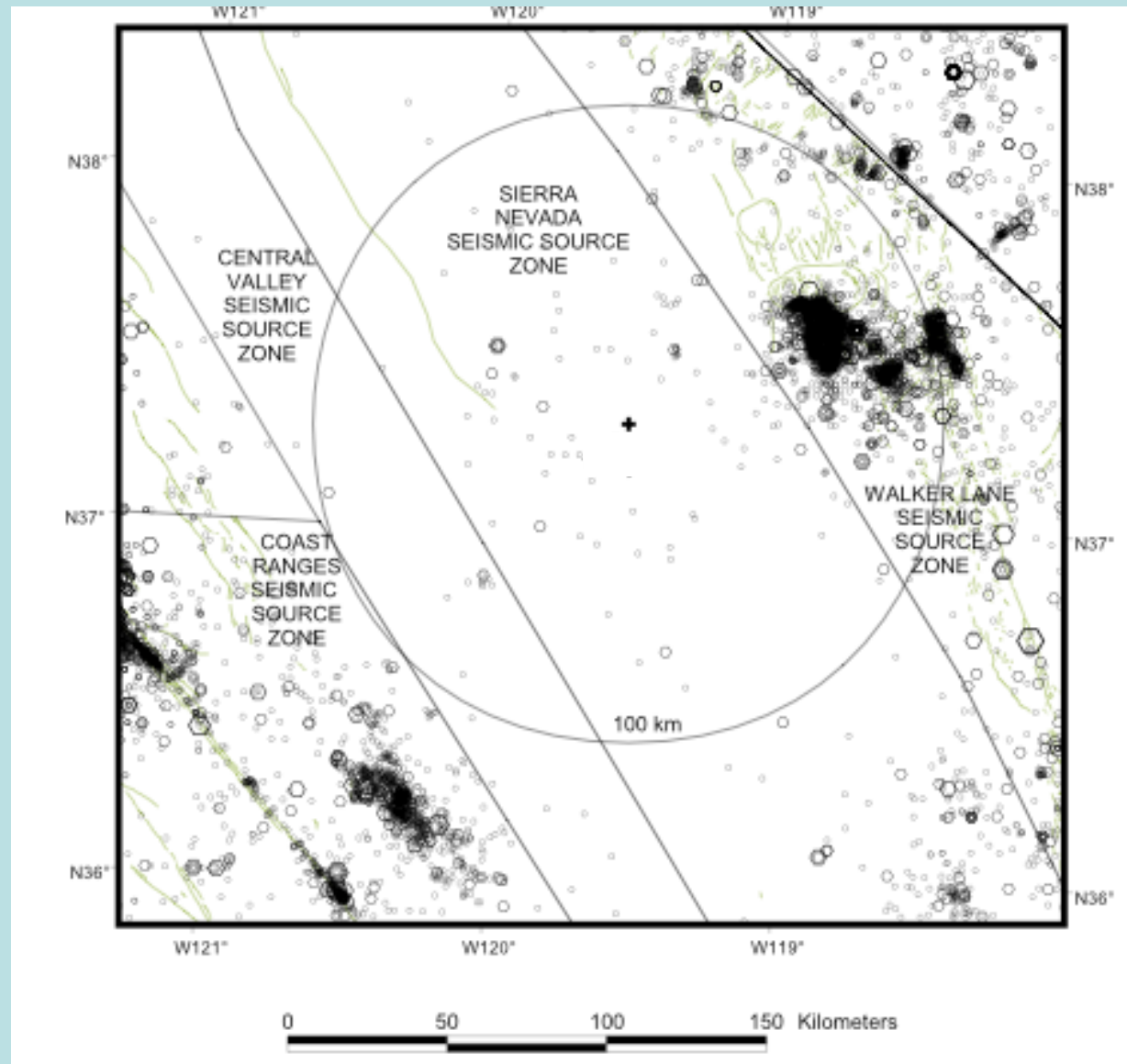
# Epistemic Uncertainty

- Due to lack of data
  - Sparse data implies large uncertainty
- In practice, not always the case
  - Estimated using alternative available models/data
    - Few available studies leads to small uncertainty  
(few alternatives available)
    - Many available studies leads to larger uncertainty  
(more alternatives available)

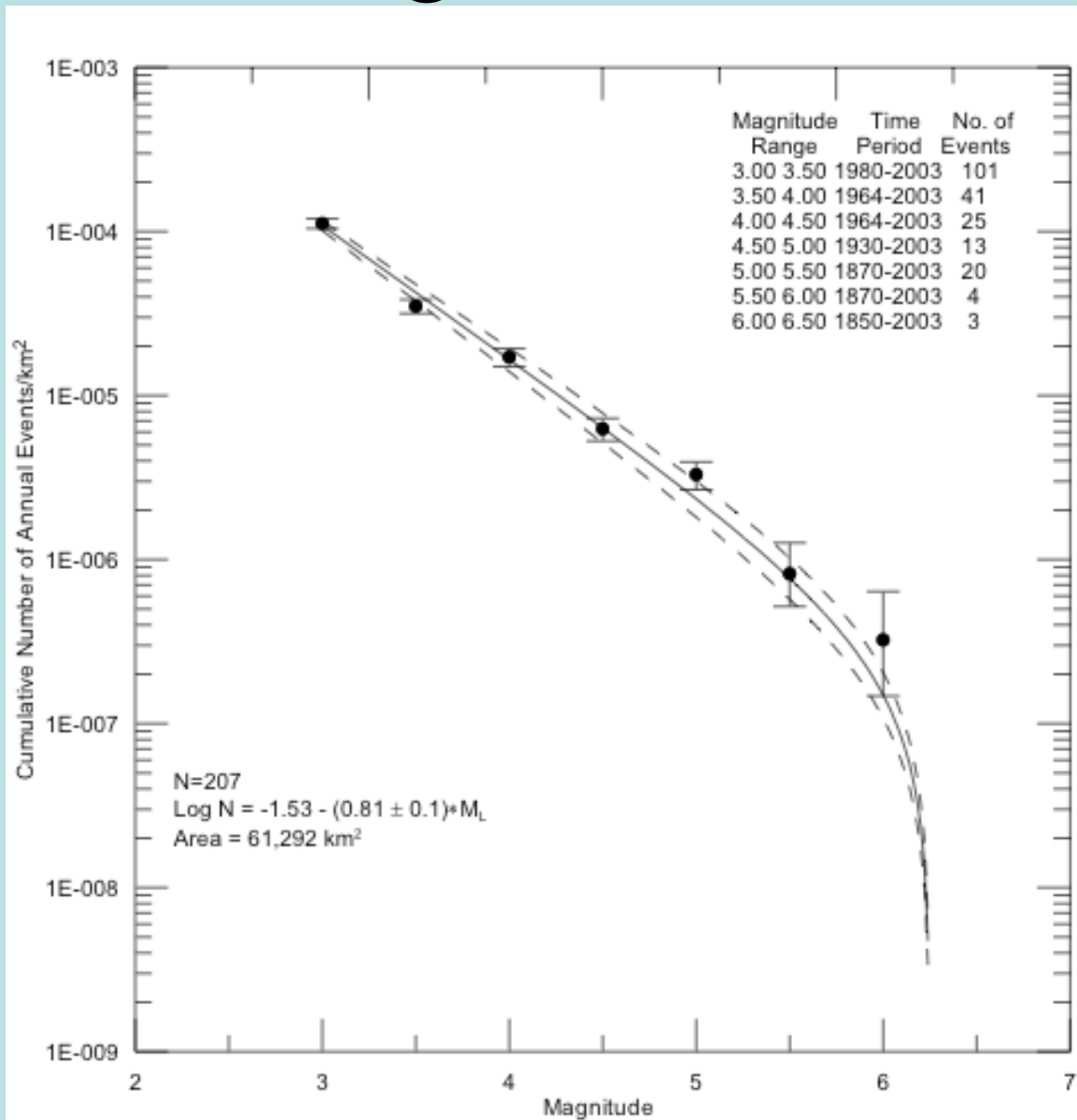
# Common Misunderstandings in PSHA

- PSHA combines ground motions from different earthquakes
  - No, PSHA ranks ground motions from different earthquakes, it does not combine ground motions
    - UHS does combine ground motions, but not required in PSHA
  - PSHA combines the chance of getting a specified level of ground motion from different earthquakes
    - - There is more than one earthquake that can lead to a specified ground motion at the site
- PSHA does not give earthquake scenarios
  - Deaggregation provides descriptions of scenarios

# Example Hazard



# Background Rate



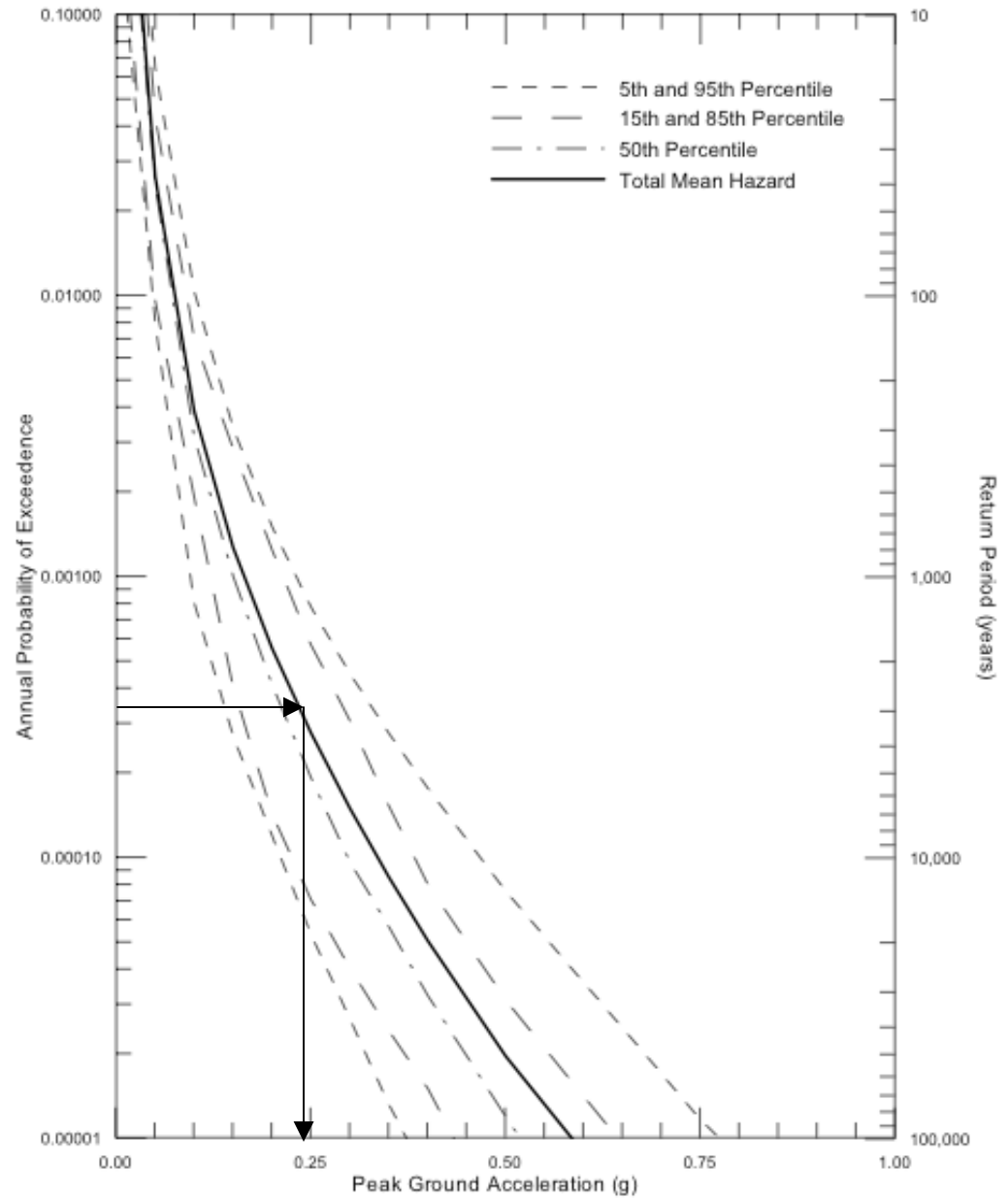
# Fault Sources

- Mean Characteristic Magnitude
  - $M = \log(\text{fault area}) + 4$
- Usually balance moment-rate on fault
  - $M_o(M) = 10^{1.5M+16.05}$
  - Moment-rate =  $\mu AS$ 
    - $\mu$  = shear modulus (3E11 dyne/cm<sup>2</sup>)
    - A = fault area in cm<sup>2</sup>
    - S = slip-rate in cm/yr

$$Eqk\ rate = \frac{Moment\ Rate}{Moment / Eqk}$$

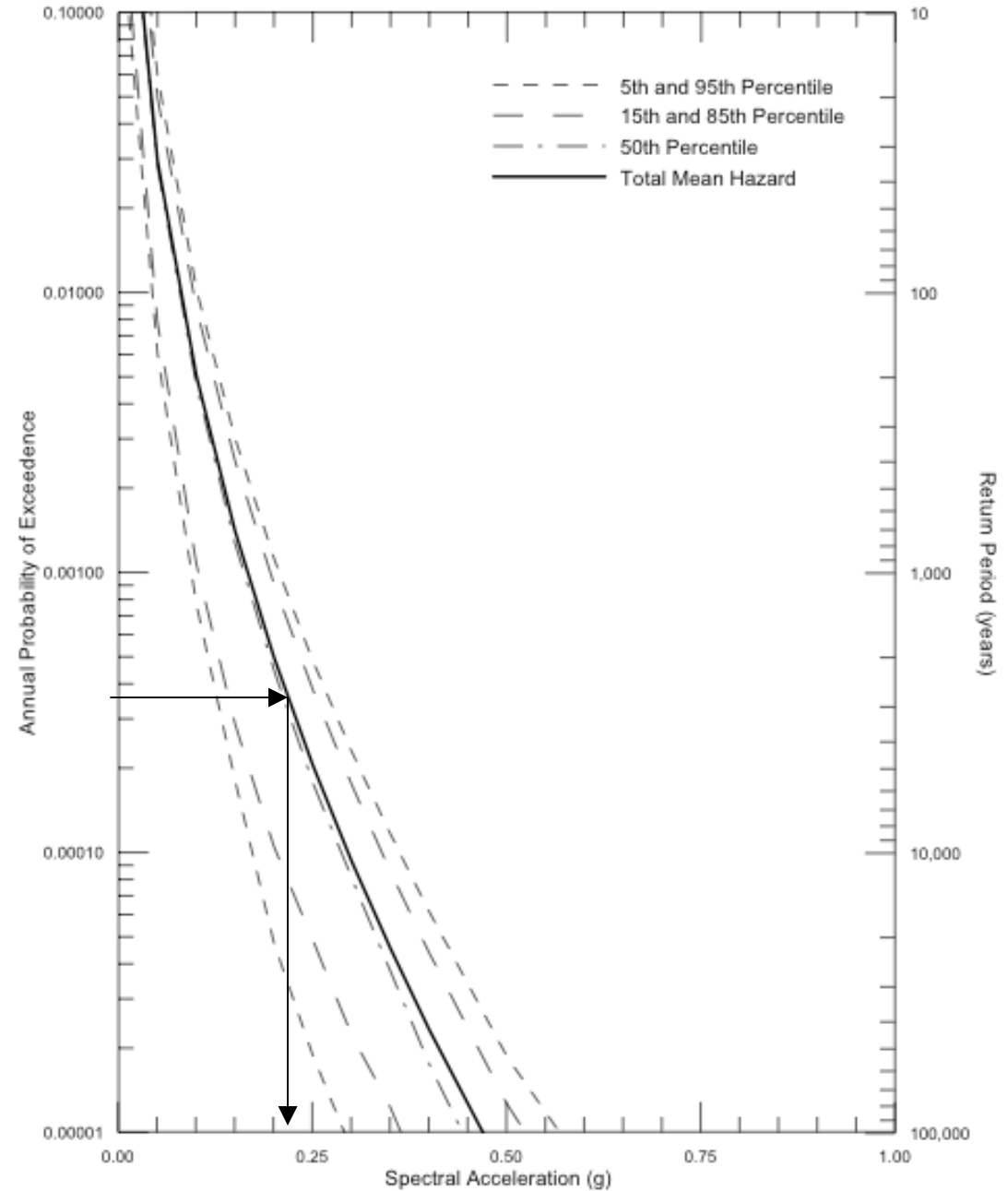
# PGA Hazard

3000 yr return period  
PGA = 0.24g

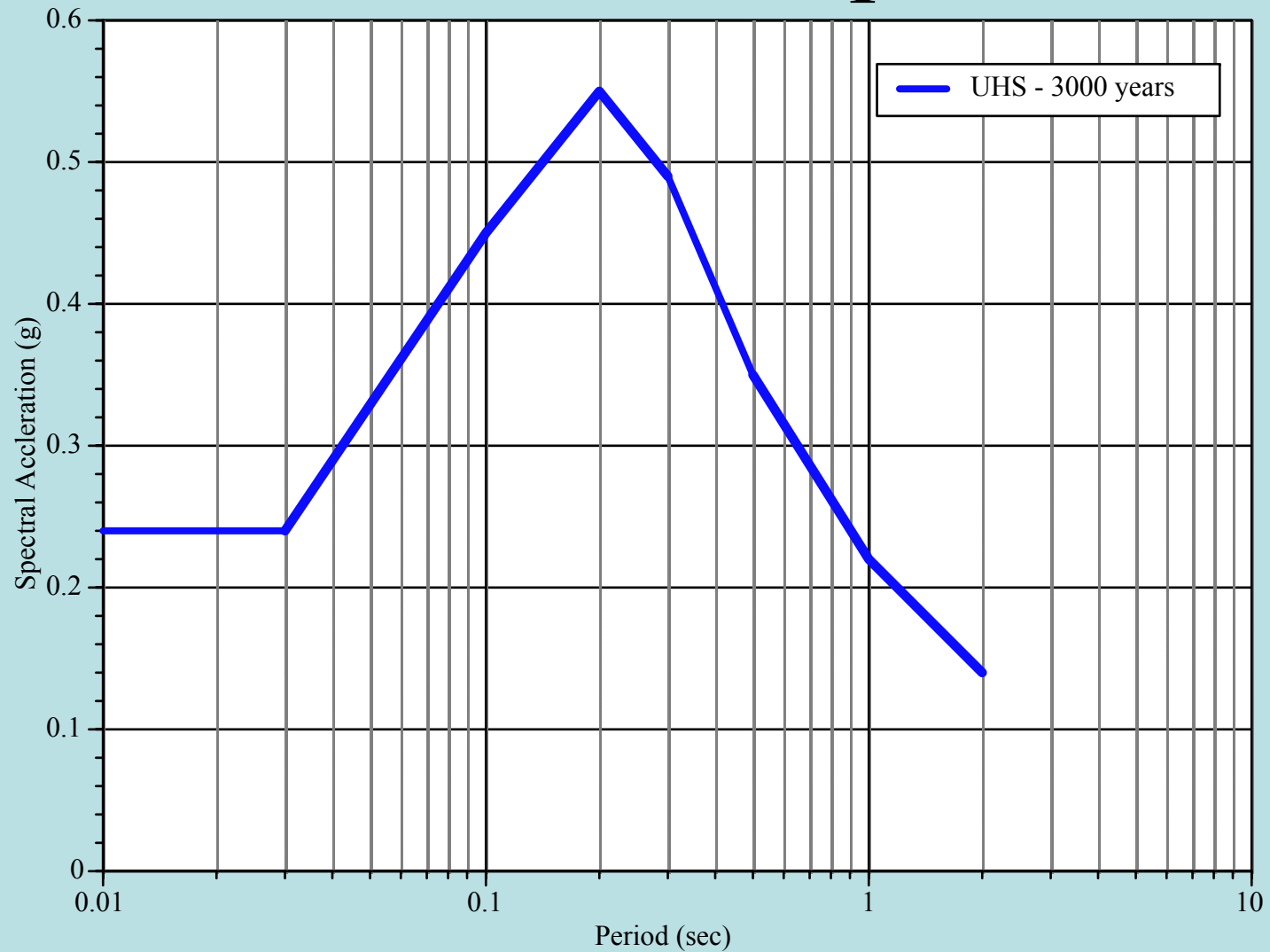


# T=1 sec Hazard

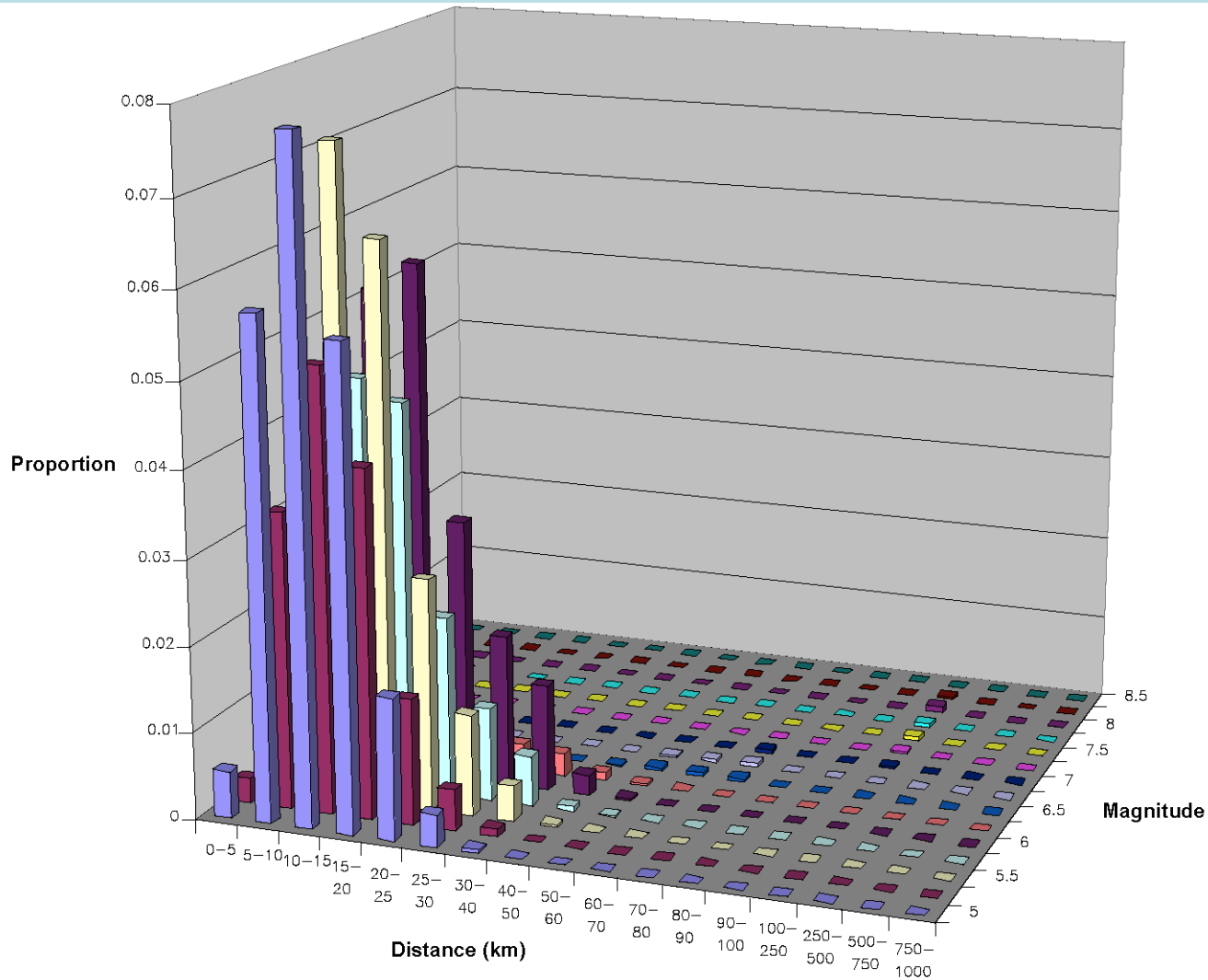
3000 yr return period  
 $S_a(T=1) = 0.22g$



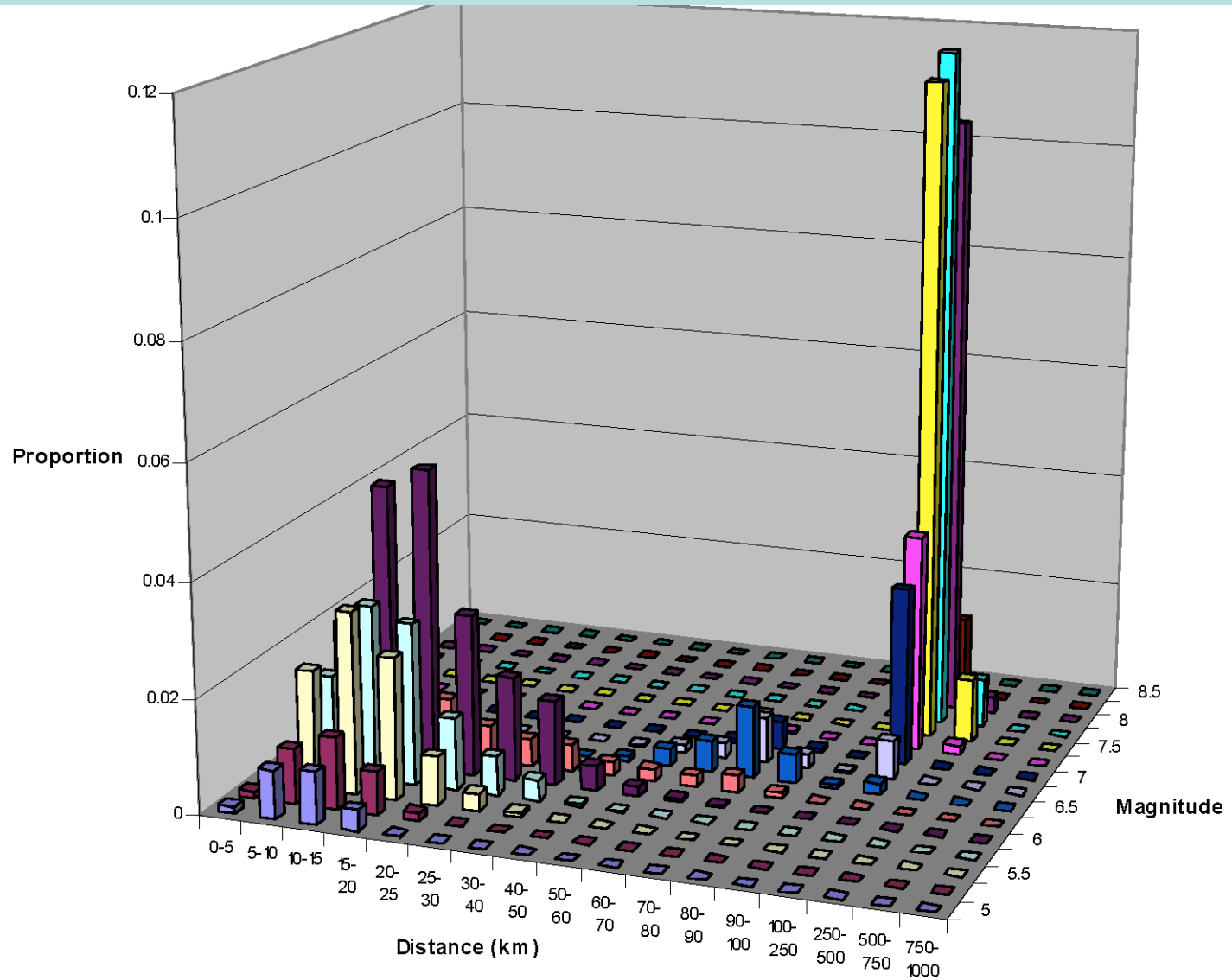
# Uniform Hazard Spectrum



# Deaggregation for $PGA=0.24g$



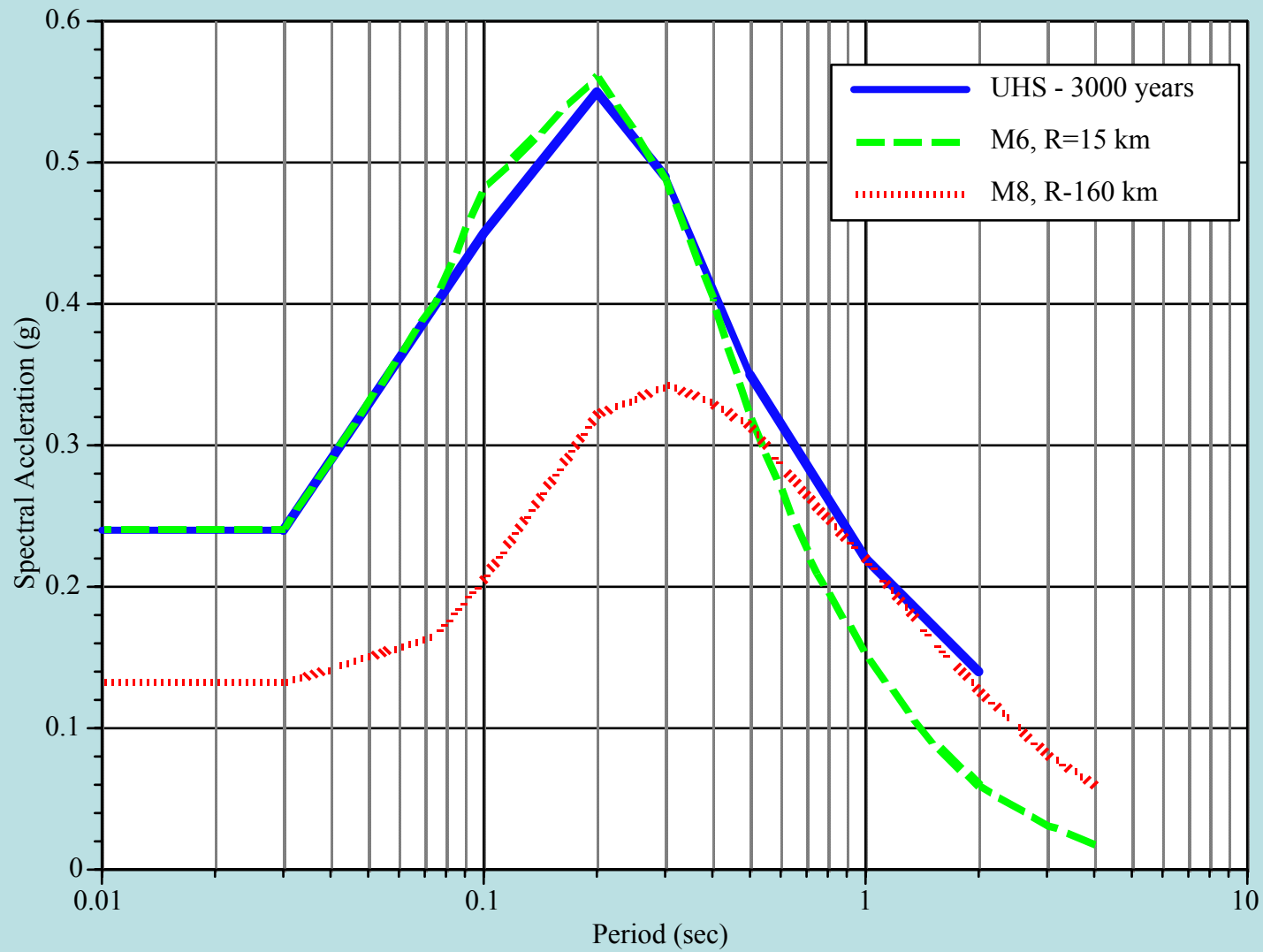
# Deaggregation for $S_a(T=1)=0.22g$



# Controlling Scenarios

- For 3000 years:
  - PGA:  $M=6.0$ ,  $R=15$  km
  - $S_a(T=1)$ :  $M=8.0$ ,  $R=160$  km

# UHS Scenarios



# UHS

- Hazard reports should provide spectra for scenarios that control the UHS
- UHS envelopes the alternative scenarios
  - used to reduce engineering analysis costs by reducing number of scenarios to consider, it is not required in PSHA
- Decision to use UHS or individual scenarios should be made by engineers involved in the analysis of structure, not by hazard analyst

# Rate of Occurrence

- Hazard curve gives rate of exceeding a ground motion
- Hazard reports should also provide a table with the rate of occurrence:

$$v(a_1 > Sa > a_2) = \text{Haz}(a_1) - \text{Haz}(a_2)$$

# Rate of Occurrence - by Mag-Dist-GM

Rate of Occurrence for a specific magnitude,  
distance and ground motion range is given by:

$$\nu(M_1 < M < M_2, R_1 < R < R_2, A_1, Sa < A_2) = \\ \text{Haz}(A_1) \text{Deag}(M_1 < M < M_2, R_1 < R < R_2, A_1) \\ - \text{Haz}(A_2) \text{Deag}(M_1 < M < M_2, R_1 < R < R_2, A_2)$$

This provides information needed for risk calculations

Ground Motion  
Prediction Equations  
(Attenuation Relationships)

# Ground Motion Regions

- Three main tectonic categories
  - Shallow crustal earthquakes in active tectonic regions
  - Shallow crustal earthquakes in stable continental regions
  - Subduction earthquakes
    - Interface
    - Intraslab

# Ground Motion Models

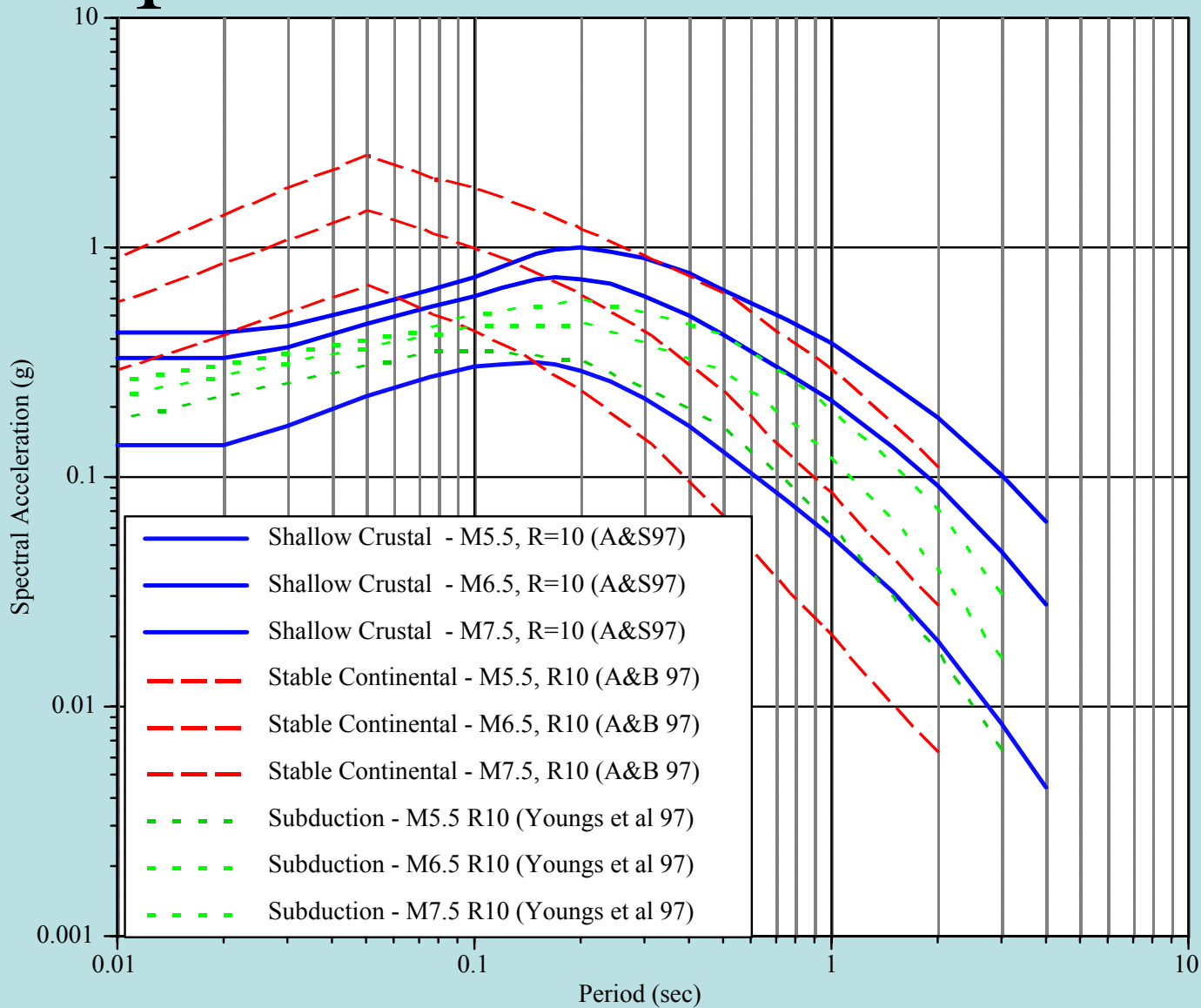
Shallow crustal earthquakes in stable continental regions

- Subduction earthquakes
  - Interface
  - Intraslab

# Ground Motion Models

- Shallow Crustal in Active Regions
  - Primarily empirically based models
- Shallow Crustal in Stable Continental Regions
  - Seismological simulations
    - Point source stochastic model
- Subduction
  - Primarily empirically based models
  - Some simulations for very large magnitudes (M9)

# Example of Tectonic Differences



# Numerical Simulations

- Separate Effects
  - Source
  - Path
  - Site
- Low frequencies ( $f < 1$  Hz)
  - Theoretical seismological models work well
- High frequencies ( $f > 1$  Hz)
  - A stochastic or empirical element is required

# Point Source Stochastic Model

- Source Parameters

M Moment magnitude

$\Delta\sigma$  Stress-drop

- Path Parameters

N Geometrical spreading coefficient

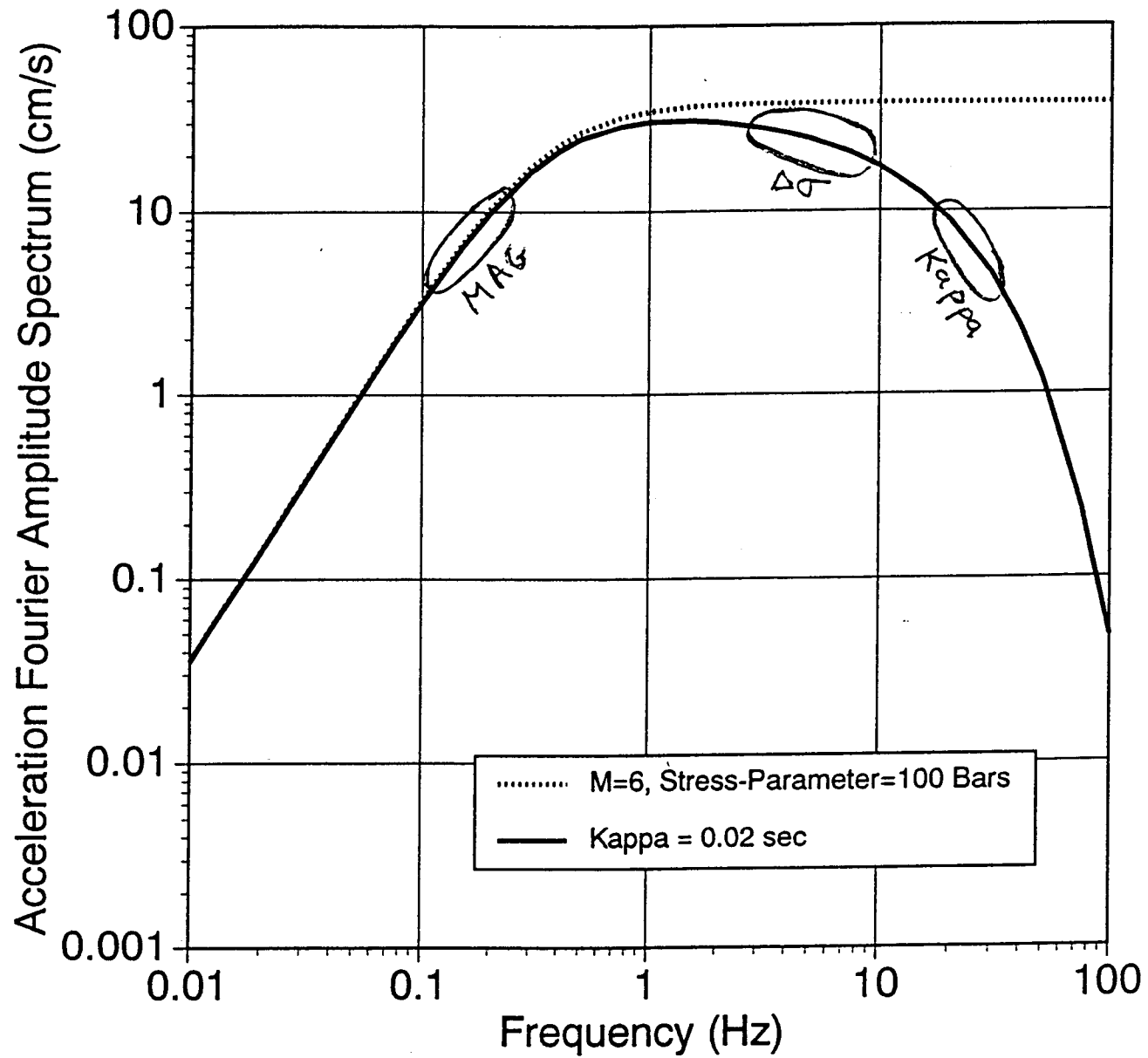
Q(f) Anelastic attenuation along ray path

- Site Parameters

$\kappa$  Accounts for damping in shallow rock

A(f) Amplification factor for the impedance contrast from source to site

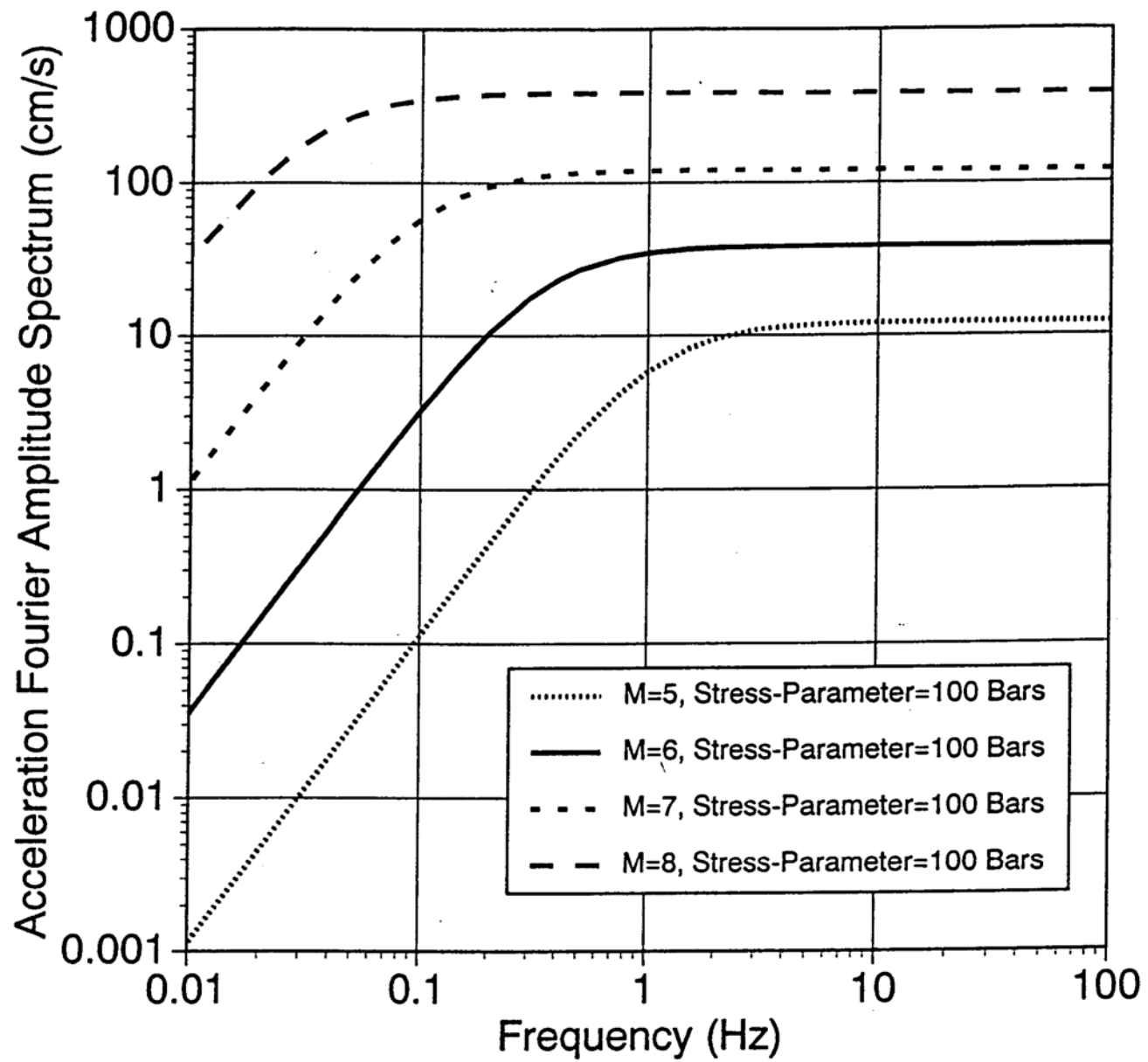
# $\omega^2$ Source Model



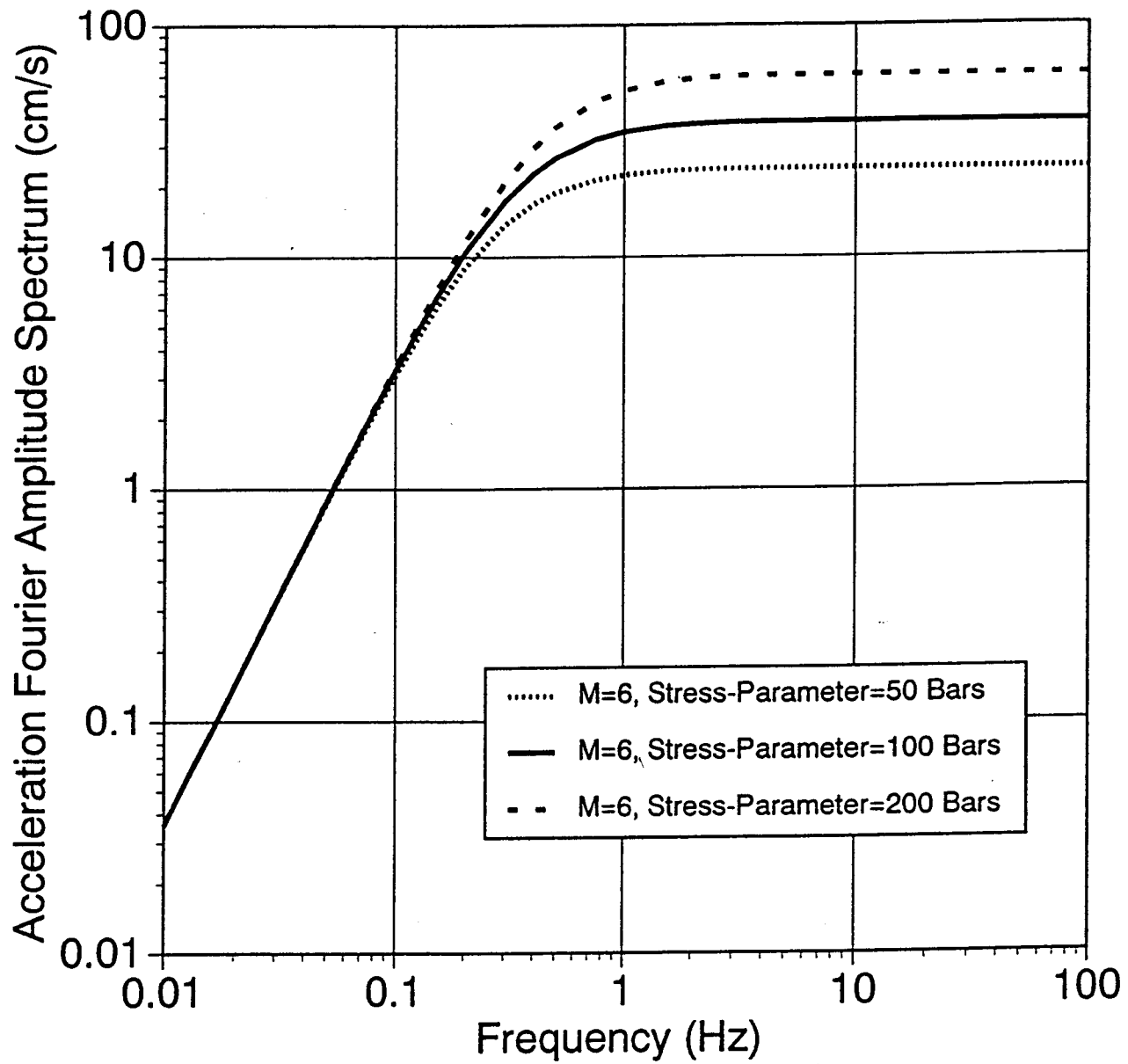
# Source Parameters

- Moment Magnitude
  - Measures total energy in earthquake
- Stress-drop
  - Measures the compactness of the source in space and/or time
  - High stress-drop leads to larger high frequency ground motions

# $\omega^2$ Source Model: Effect of Magnitude



# $\omega^2$ Source Model: Effect of Stress-Parameter

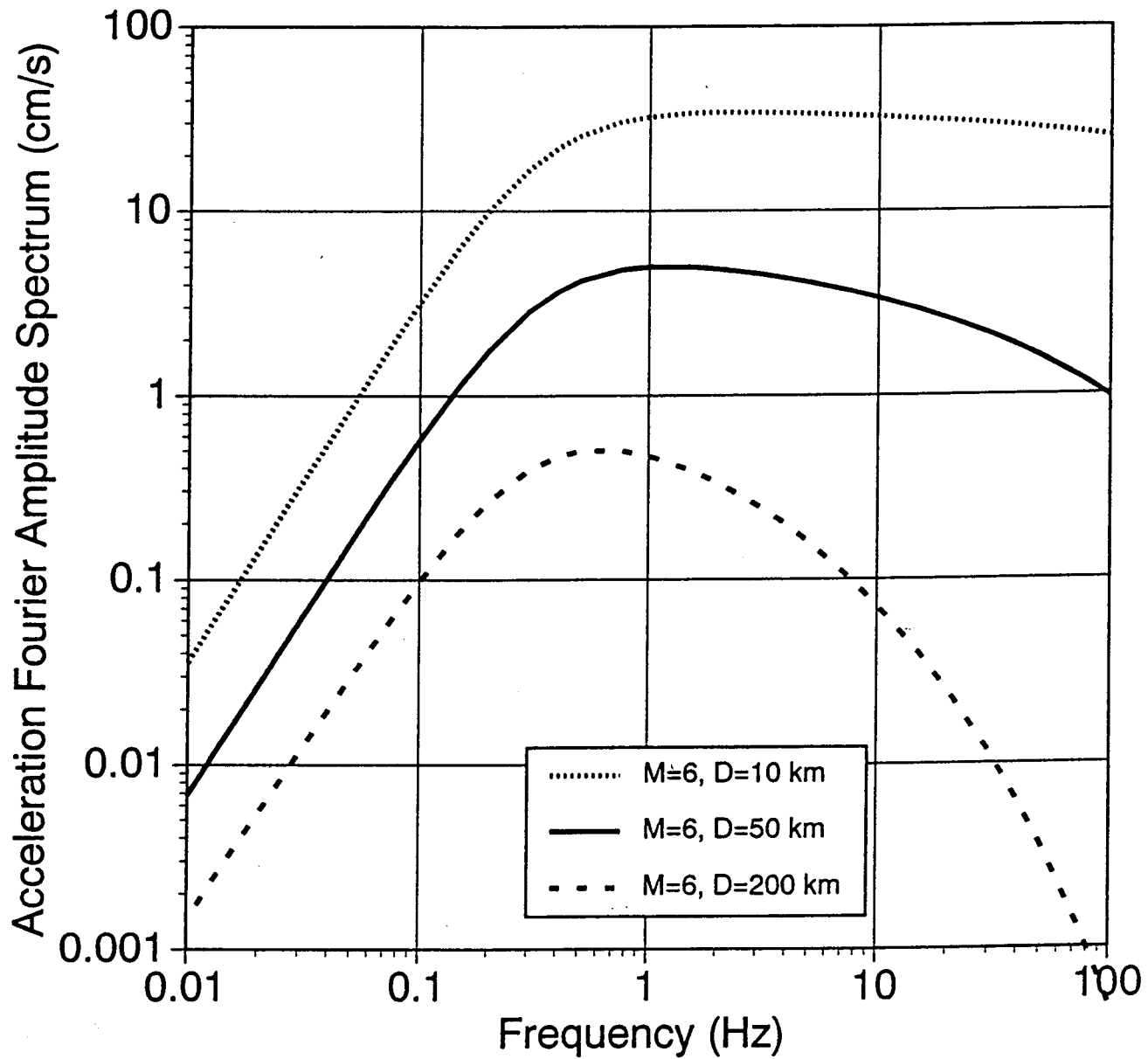


# Path Parameters

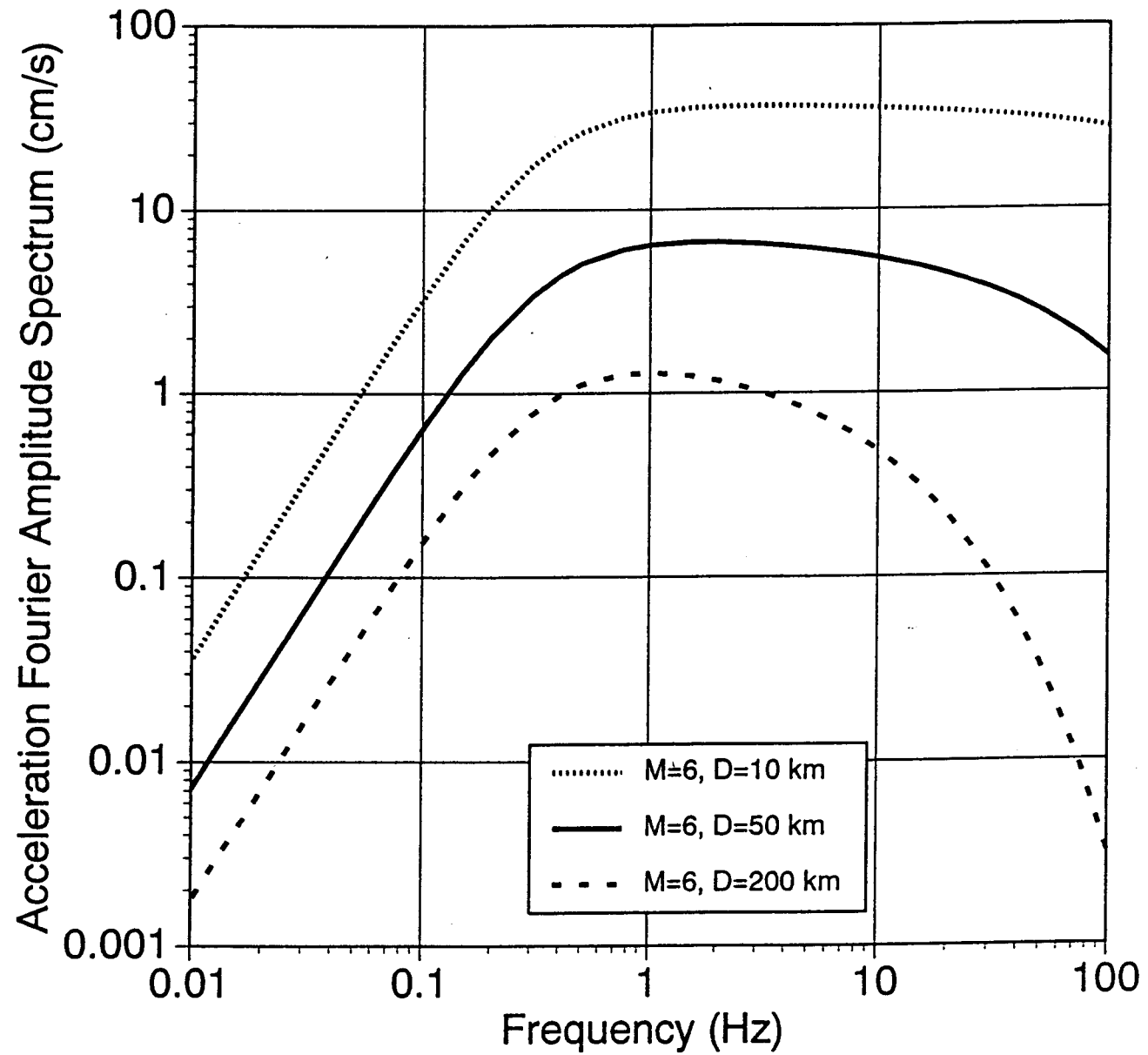
$$R^{-N} \exp\left(\frac{-\pi f R}{V_s Q(f)}\right)$$

- Geometrical Spreading (N)
  - Describes distance attenuation from a point source
- Q
  - In practice, Q accounts for all source of attenuation not modeled by the geometrical spreading
  - Q model must be consistent with geometrical spreading model

$\omega^2$  Source Model: Effect of Q (WUS)



$\omega^2$  Source Model: Effect of Q (EUS)

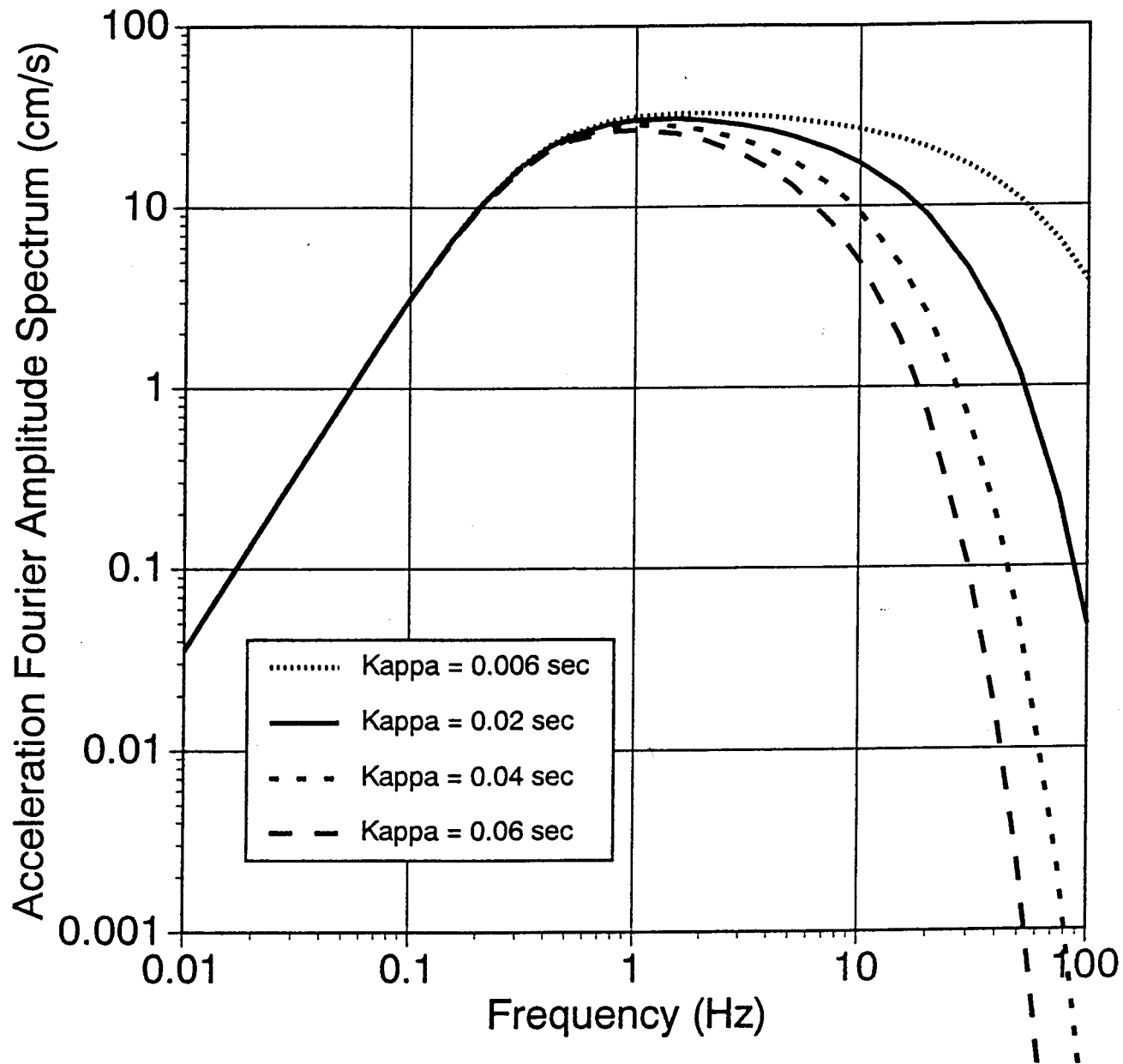


# (rock) Site Parameters

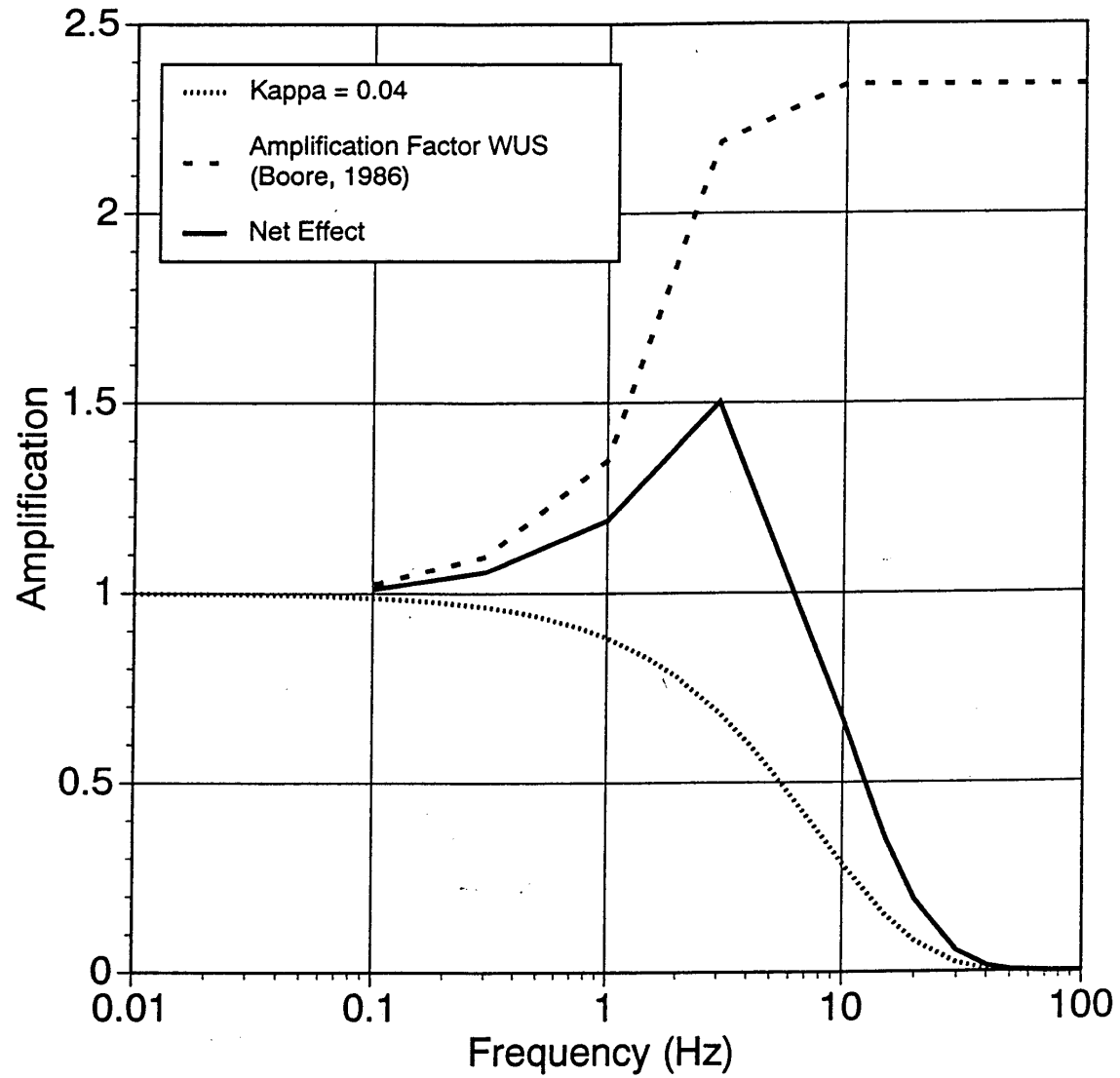
$$A(f) \exp(-\pi\kappa f)$$

- $A(f)$ 
  - Accounts for amplification due to impedance contrast in crust
- $\kappa$ 
  - Accounts for attenuation of the high frequency (related to material damping in the shallow crustal rock)
  - In practice, accounts for high frequency differences between point source model and observations
    - May be due to source effects as well as site effects
  - Same effect also modeled by  $f_{\max}$

# $\omega^2$ Source Model: Effect of Kappa



### Example of Site Effect Scaling of Fourier Amplitude Due to Amplification Factor and Kappa



# Shallow Crustal Earthquakes in Active Tectonic Regions

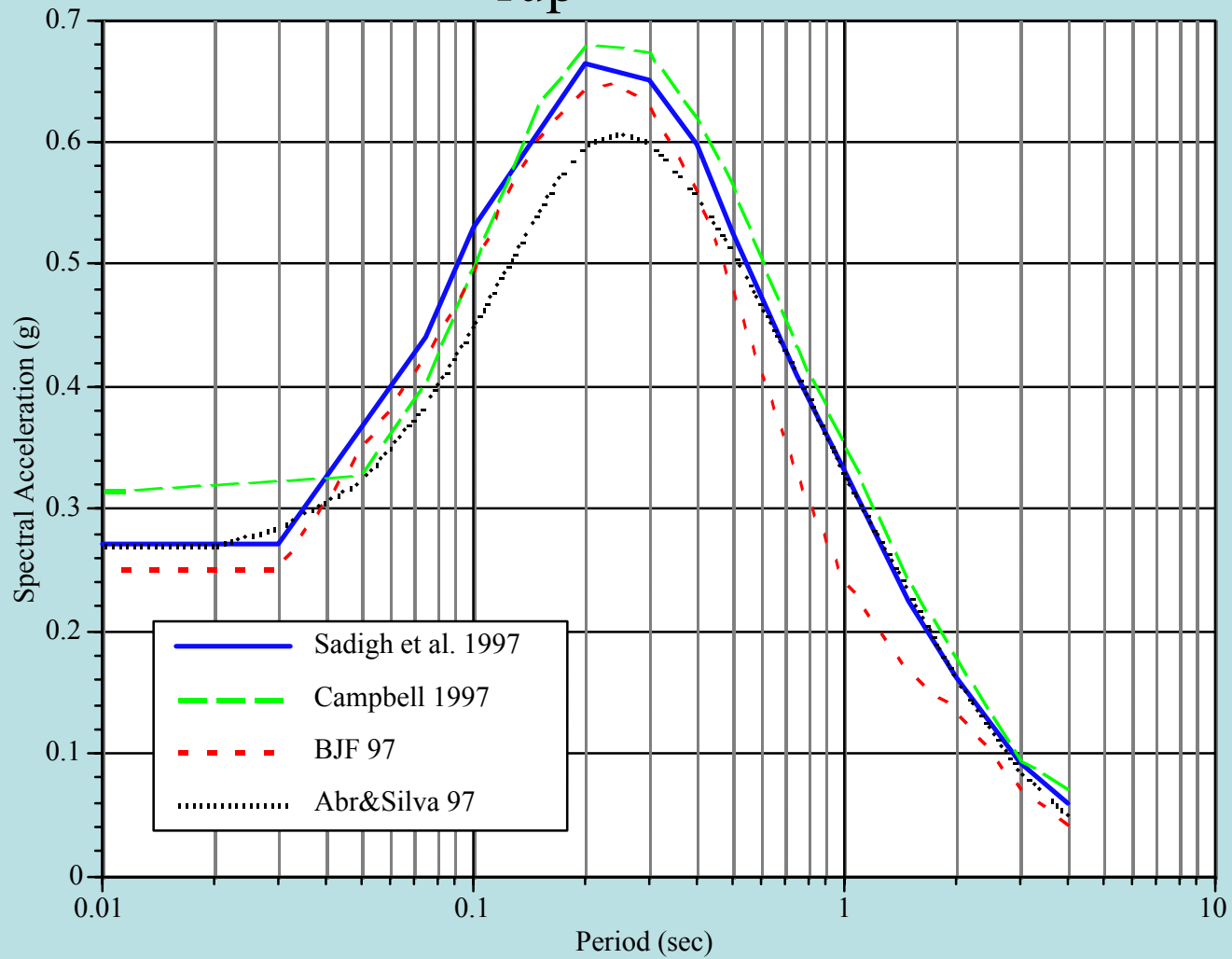
- Empirically Based Models
  - Summary of models in 1997 Seismological Research Letters
    - Abrahamson & Silva (1997)
    - Boore et al (1997)
    - Campbell (1997)
      - Updated model by Campbell and Bozorgnia (2002)
    - Sadigh et al (1997)
    - Spudich et al (1997)
      - - for extensional regimes
  - Other Models commonly used
    - Idriss (1994)

# Different Parameters Used

Model	Distance	Site	Style-of-faulting	Other Param
A&S	Rrup	Rock Soil	SS, RV/OBL, RV	HW
BJF	RJB	VS30	SS, RV	
Campbell	Rseis	Hard-rock Soft-rock Soil	SS RV+RV/OBL	Depth to basement rock
Idriss	Rrup	Rock Soil	SS, RV/OBL, RV	
Sadigh	Rrup	Rock Deep Soil	SS, RV/OBL, RV	

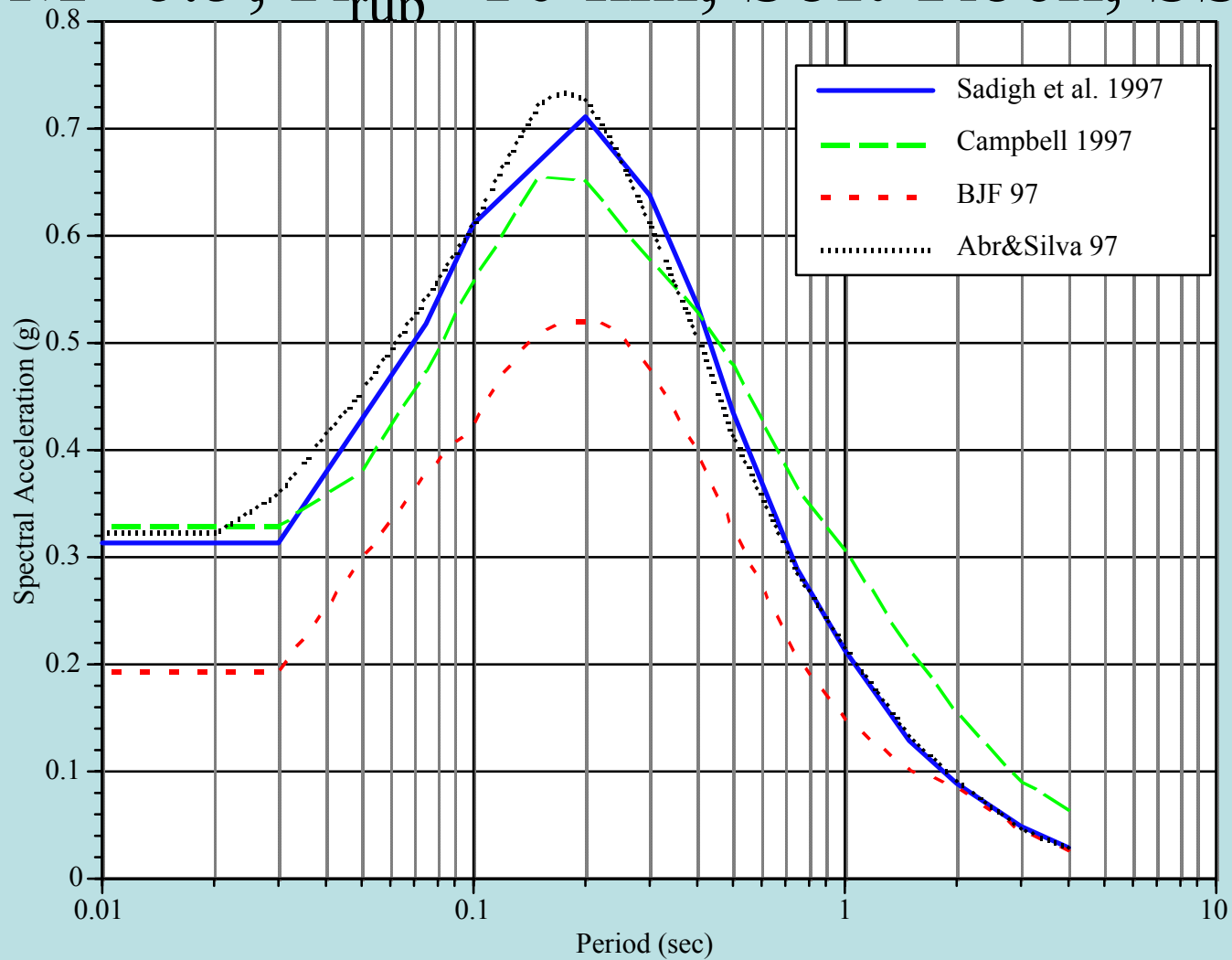
# Comparison of Median Sa

$M=6.5$ ,  $R_{rup}=10$  km, SS, Soil



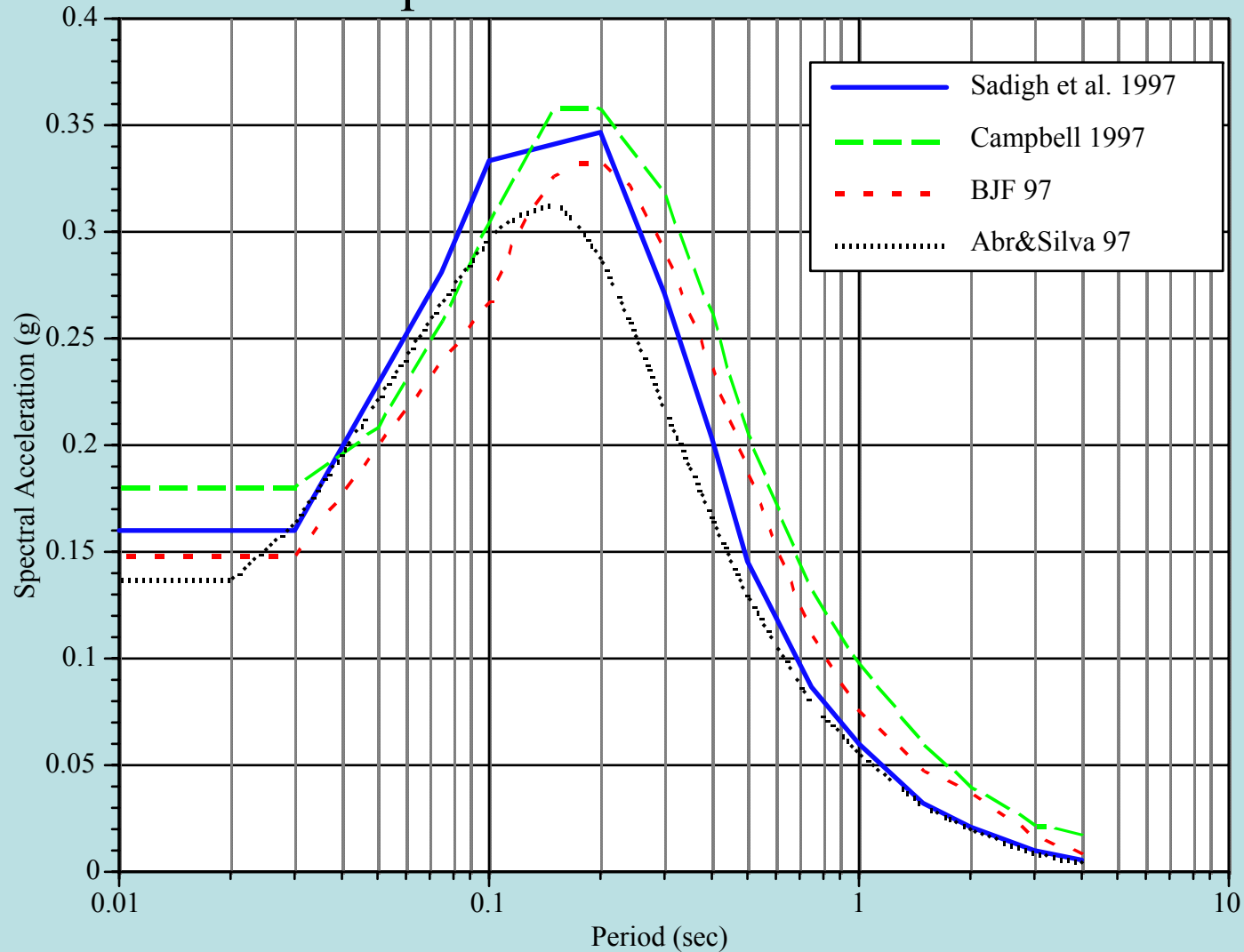
# Comparison of Median Sa

$M=6.5$ ,  $R_{rup}=10$  km, Soft-Rock, SS



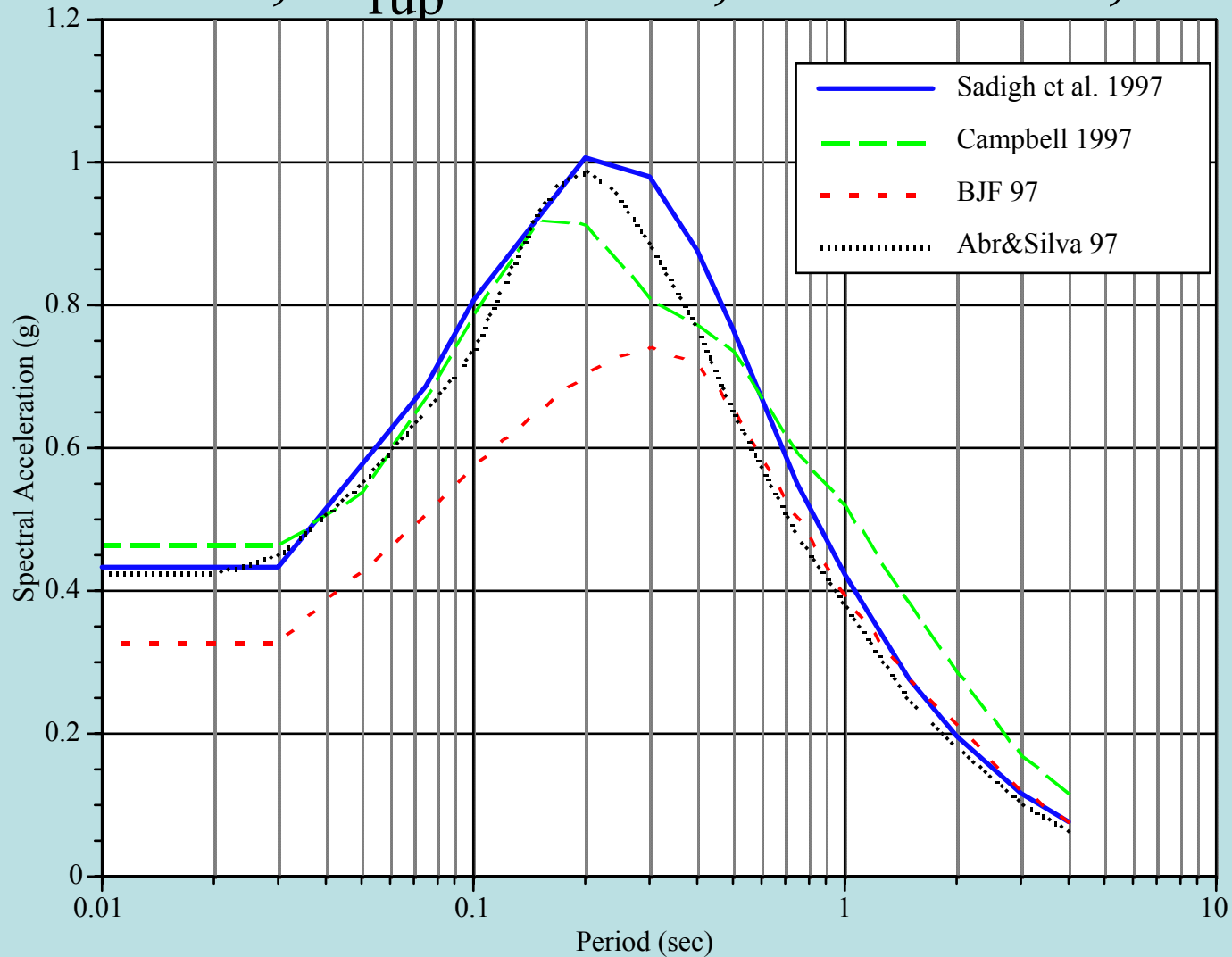
# Comparison of Median Sa

$M=5.5$ ,  $R_{rup}=10$  km, Soft-Rock, SS



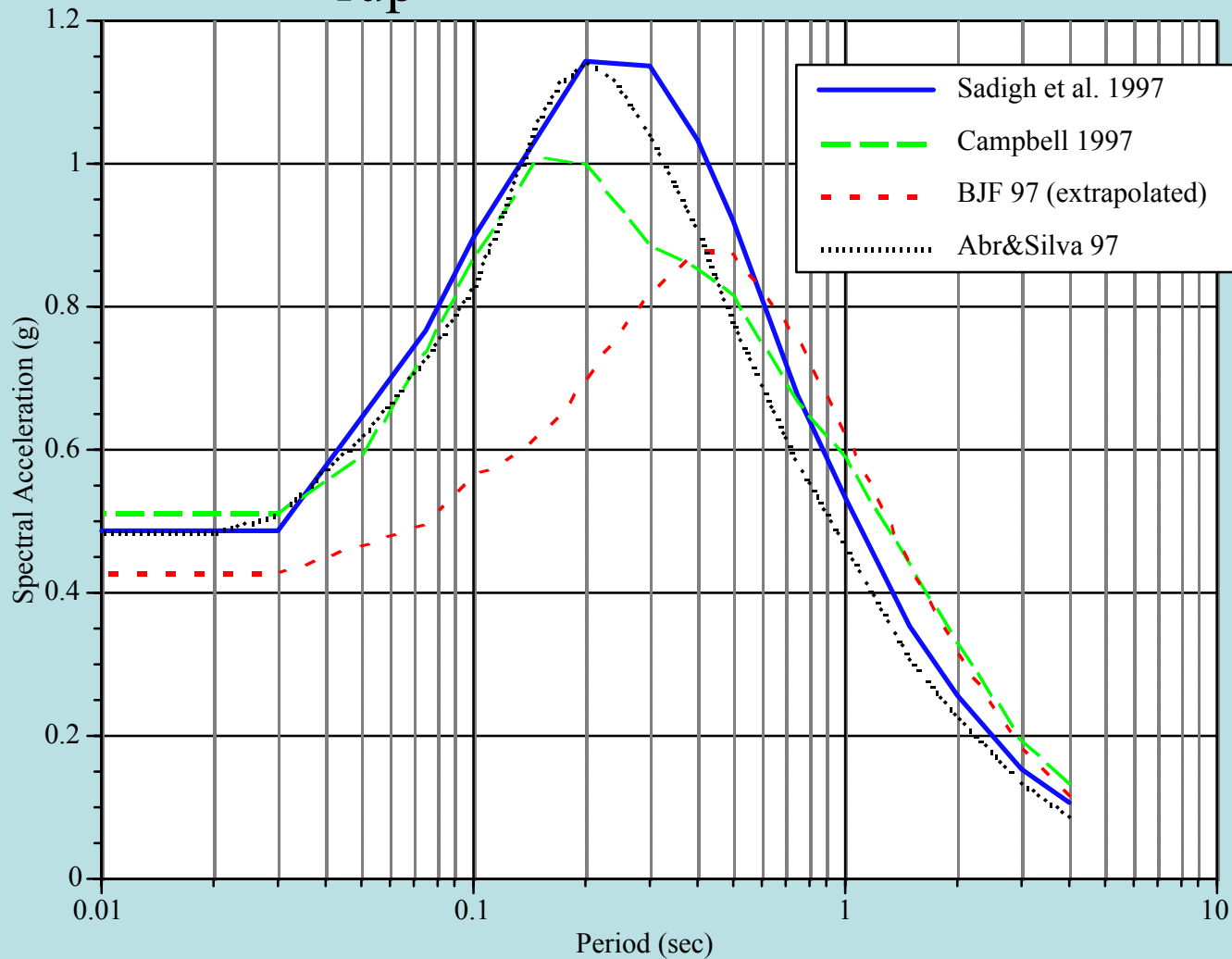
# Comparison of Median Sa

$M=7.5$ ,  $R_{rup}=10$  km, Soft-Rock, SS

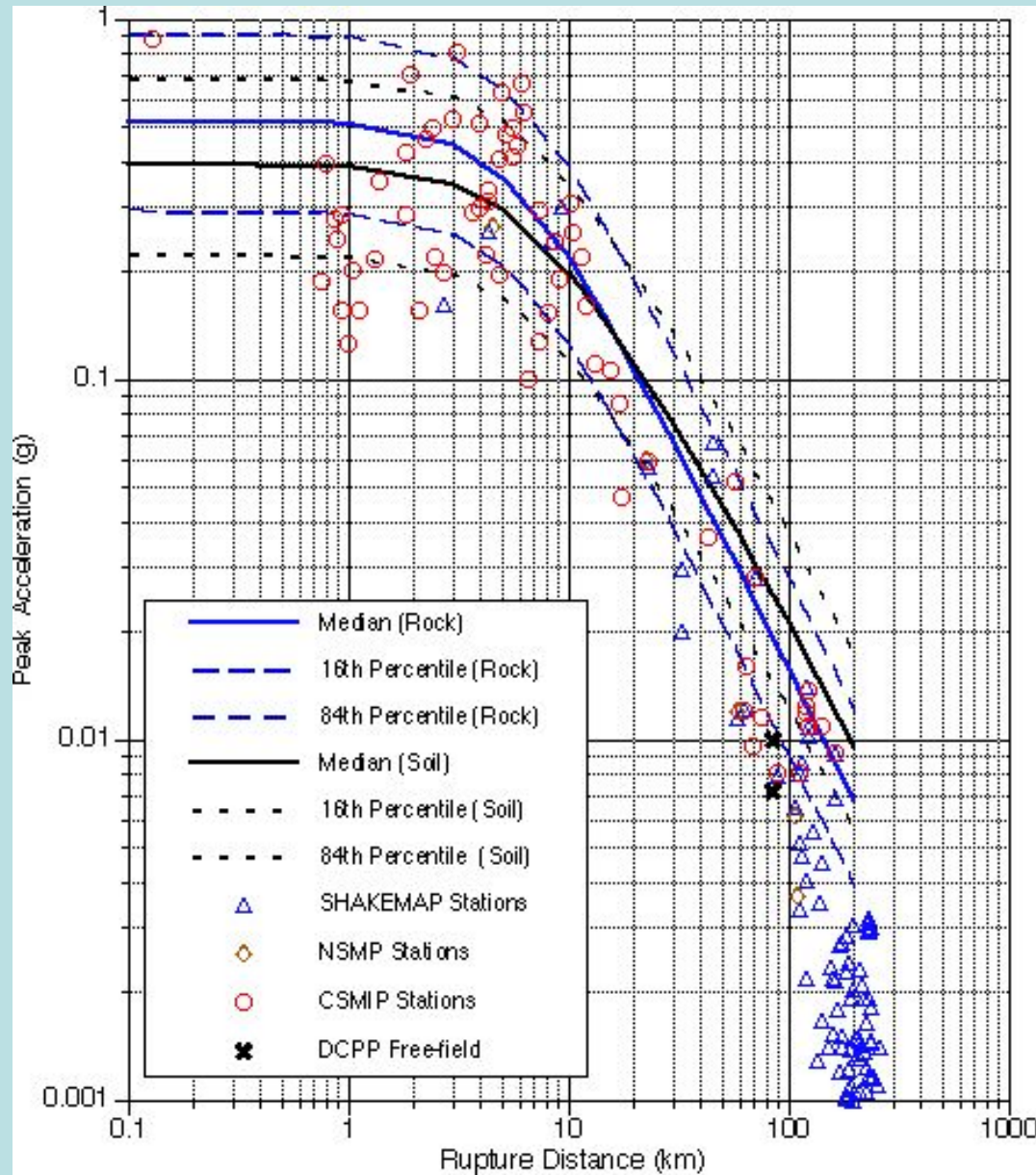


# Comparison of Median Sa

$M=8.0$ ,  $R_{rup}=10$  km, Soft-Rock, SS



# 2004 Parkfield Attenuation Comparison with A&S 97



# Next Generation Attenuation (NGA) Project

- Joint Project
  - PEER/Lifelines Program
    - PEER
    - Caltrans
    - CEC
    - PG&E
  - SCEC
  - USGS

# NGA Project

- NGA-E (empirical)
  - New ground motion models based primarily on empirical data
  - Use analytical models (seismological and geotechnical) to guide extrapolation outside of empirical data
    - Results used only in terms of scaling
- NGA-H (hybrid)
  - New ground motion models based on both empirical data and numerically simulated data
    - Uses the values of the ground motion from the simulations, not just scaling

# NGA-E Project

- Ground motion models for crustal earthquakes in California
- Must be applicable to all design cases in California (excludes subduction)
  - Ground Motion Parameters:
    - Horizontal components (Ave Horiz, FN, and FP)
    - PGA, PGV, PGD
    - Pseudo spectral acc at 5% damping: 0-10 sec
  - Applicable Magnitude Range:
    - 5.0 - 8.5 (SS)
    - 5.0 - 8.0 (RV)
  - Applicable Distance Range:
    - 0 - 200 km
  - Fault Types
    - Strike-slip, Reverse, Normal

# Ground Motion Model Developers

- Abrahamson & Silva
- Boore and Atkinson
- Campbell & Bozorgnia
- Chiou & Youngs
- Idriss

# Empirical Data Set

- Common data set provided to developers
  - 175 earthquakes
  - 3548 recordings
- Each developer selects applicable subset of data
  - Justify the exclusion of earthquakes and recording sites
  - Document any modification of independent parameters from PEER recommended values
  - Ground motion values are not modified (e.g. common set used by all developers), but developers select applicable period range of each recording

# Developer Scope

- Evaluate new predictive parameters
  - Directivity parameters
  - Hanging wall / foot wall
  - Static stress-drop
  - Asperity depth
  - Depth to “rock” ( $V_s$  1.0, 1.5, or 2.0 km/s)
  - Basin parameters
- Each developer decides if new predictive parameter is to be included in their model
  - Justify the selection or rejection of each parameter evaluated

# Independent Parameters

- Magnitude definition
  - All developers use moment magnitude
- Distance definition
  - Developers select appropriate distance metric
- Site classification
  - Developers select site classification
    - Clear definition of site classes
  - Provide a translation scheme to NERHP categories

# Supporting Simulations

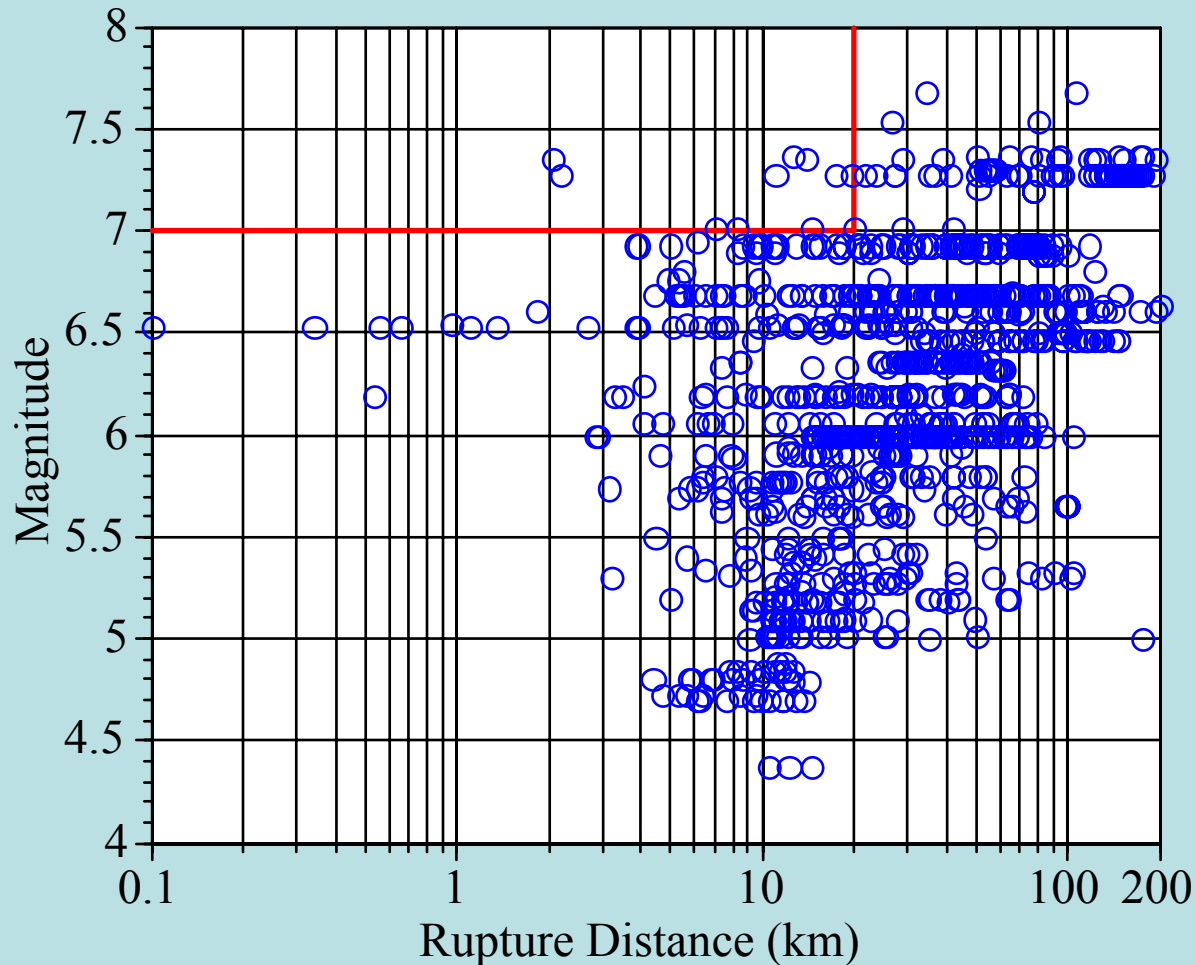
- 1-D Rock simulations
- 3-D basin simulations
- Isochron simulations
- Site amplification calculations

# 1-D Rock simulations

- Simulations by three groups
  - Zeng
  - URS
  - Pacific Engineering
- Simulations can be used for:
  - Magnitude scaling above M7
  - Distance scaling 0-15 km and 70-200 km
  - Period extrapolation to 10 seconds
  - Directivity scaling
  - Hanging wall footwall scaling
  - Asperity depth (shallow vs buried slip)
  - Stress-drop scaling

# Empirical Models, Pre-1995

## Shallow Crustal Earthquakes in Active Regions



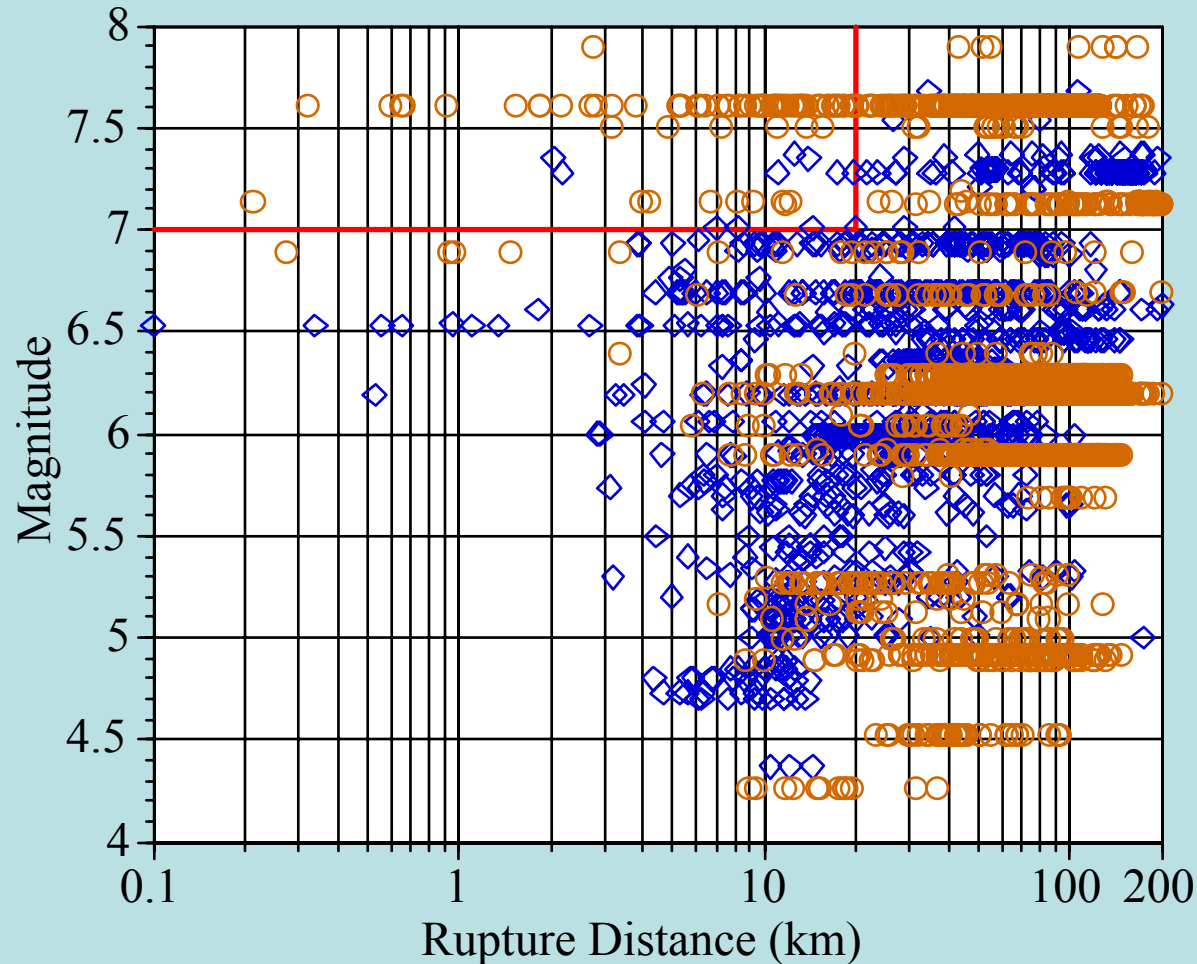
- Main basis for current models
- Few data from  $M > 7$ ,  $\text{Dist} < 20$  km
- Extrapolation of scaling for  $M 6-7$  up to  $M 8$

# Recent Large Magnitude Earthquakes with Ground Motion Data

- 1999 Kocaeli, Turkey (M7.5)
- 1999 Chi-Chi, Taiwan (M7.6)
- 1999 Duzce, Turkey (M7.1)
- 1999 Hector Mine, CA (M7.1)
- 2002 Denali, Alaska (M7.9)

# Empirical Models, Pre-1995

## Shallow Crustal Earthquakes in Active Regions

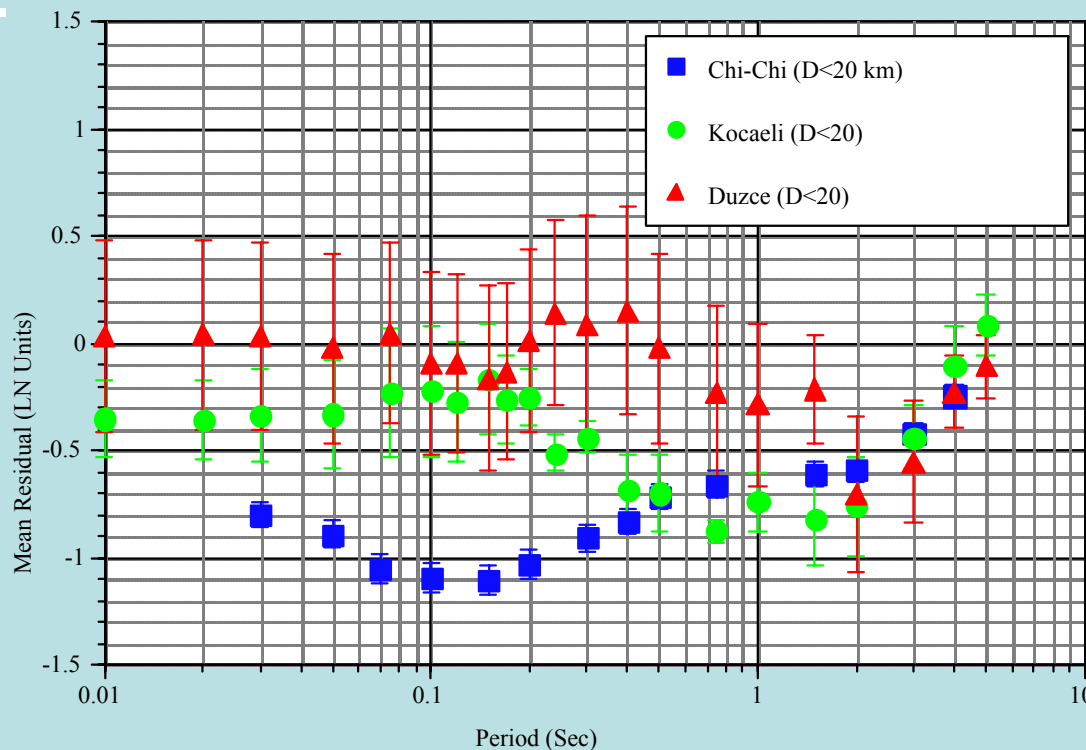


- Basis for new models
- Additional data for  $M > 7$ ,  $\text{Dist} < 20 \text{ km}$  mainly foreign

# Recordings Close to Large Earthquakes

	$M \geq 7.0$ $D \leq 20$ km	$M \geq 7.0$ $D \leq 10$ km	$M \geq 7.0$ $D \leq 5$ km
Prior to 1999	11	4	2
1999 Kocaeli	6	3	2
1999 Chi-Chi	63	32	14
1999 Duzce	6	2	1
1999 Hector Mine	1	0	0
2002 Denali	1	1	1

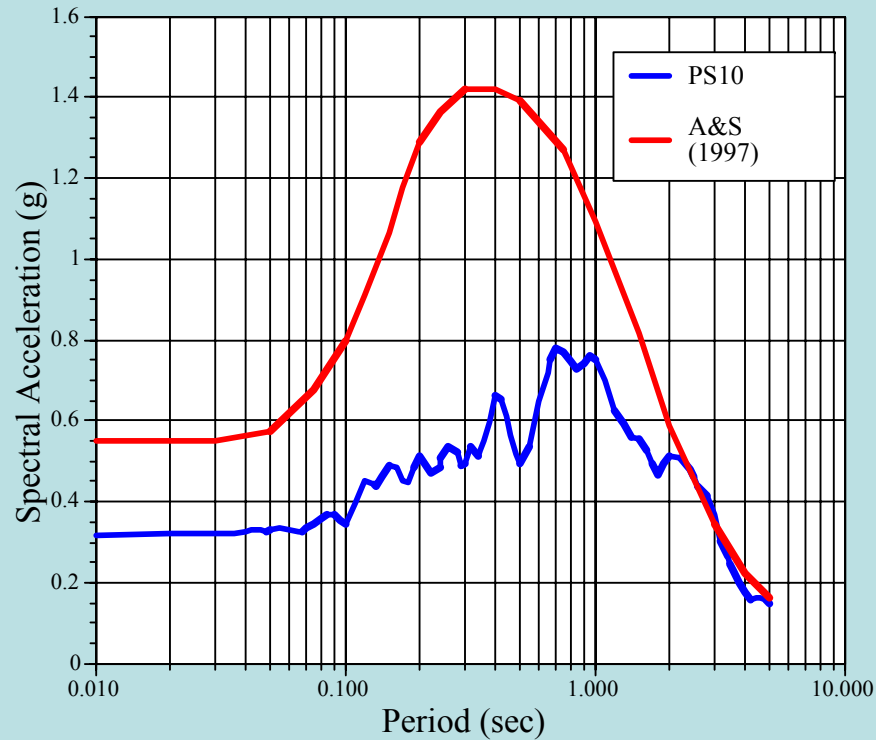
# Comparison of New Data with Current Ground Motion Models



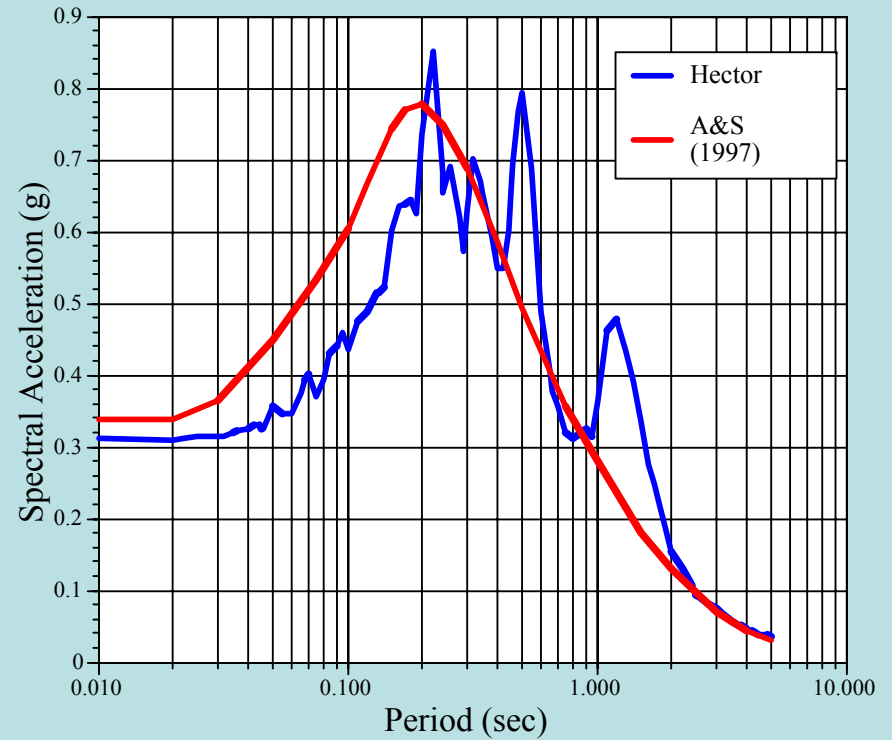
- Close-in data are over-predicted by current models
- Denali & Hector Mine have just one recordings < 20 km

# Comparison of New Data with Current Ground Motion Models

## Denali



## Hector Mine



# Issues for Ground Motion Data from Recent Large Earthquakes

- Are the data from these non-California earthquakes applicable to California?
  - Regional differences in stress-drop, crustal structure?
- Were these earthquakes just randomly weaker?
  - With large variability, will future earthquakes have higher ground motions?
- Are the recorded data representative of the overall shaking?
  - Were the locations of ground motion recorders biased to lower than average ground motions?

# NGA Project

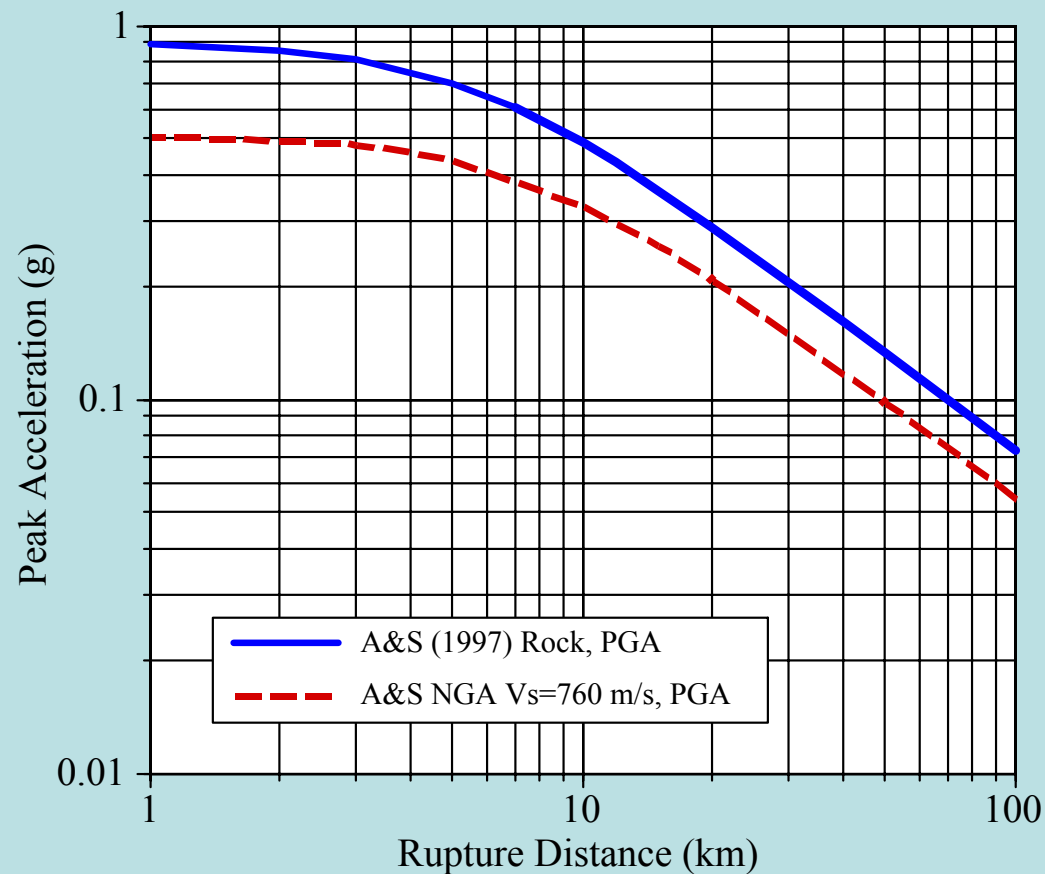
- Develop new ground motion attenuation relations for shallow crustal earthquakes in active regions
  - Sponsors
    - PEER, Caltrans, PG&E, Calif Energy Commission
  - Partners
    - SCEC, USGS, CGS
  - Multiple ground motion model developer teams
    - Abrahamson & Silva
    - Boore and Atkinson
    - Campbell and Bozorgnia
    - Chiou and Youngs
    - Idriss

# Site Classification for NGA Project

- Major effort to estimate  $V_{S30}$  for strong motion sites
  - Studies to measure  $V_{S30}$
  - Correlation of  $V_{S30}$  with surface geology
- Allows the NGA models to use  $V_{S30}$  rather than generic soil categories

# Example NGA Result

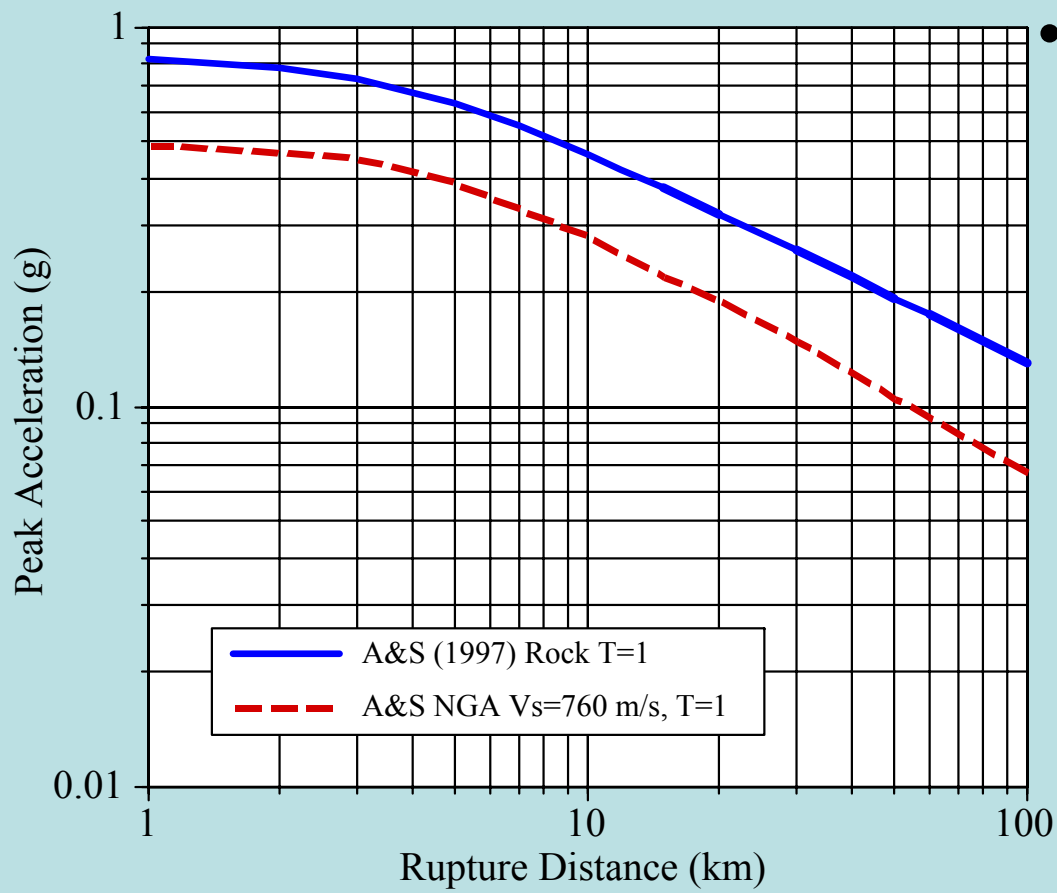
## M8, PGA, SS, rock



- 12% reduction due to Vs30 differences (760 m/s vs generic rock)

# Example NGA Result

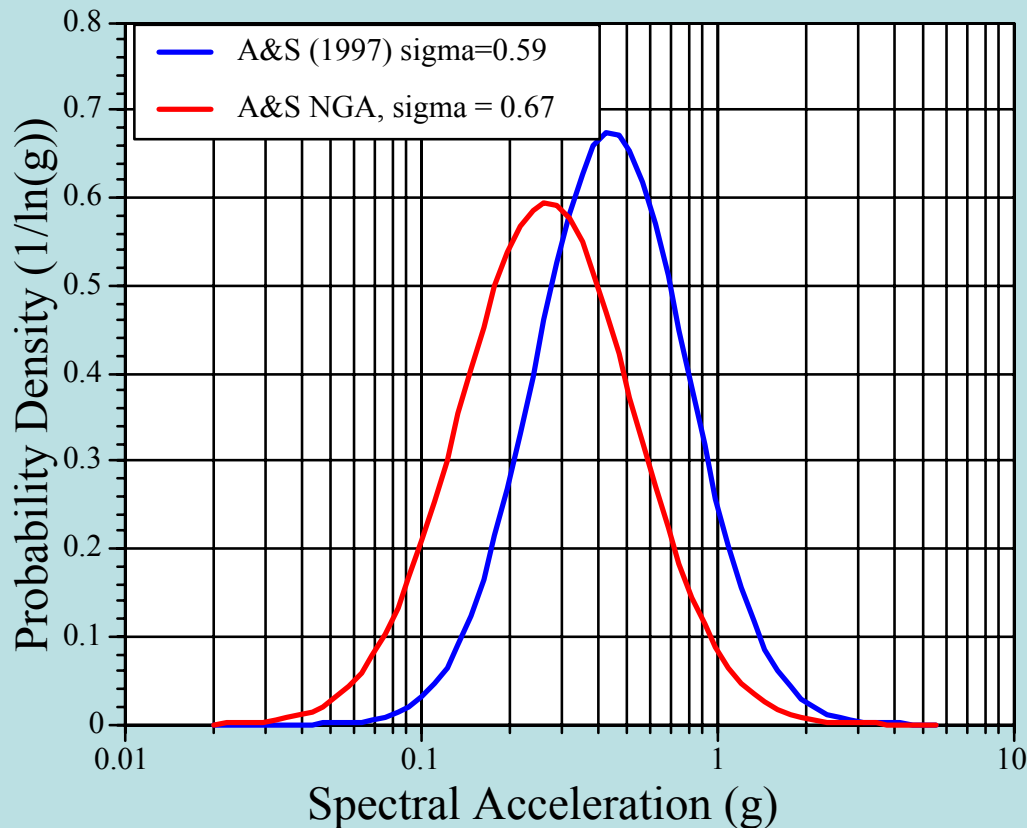
## M8, T=1 sec, SS, rock



- 25% reduction due to Vs30 differences (760 m/s vs generic rock)

# Example NGA Variability

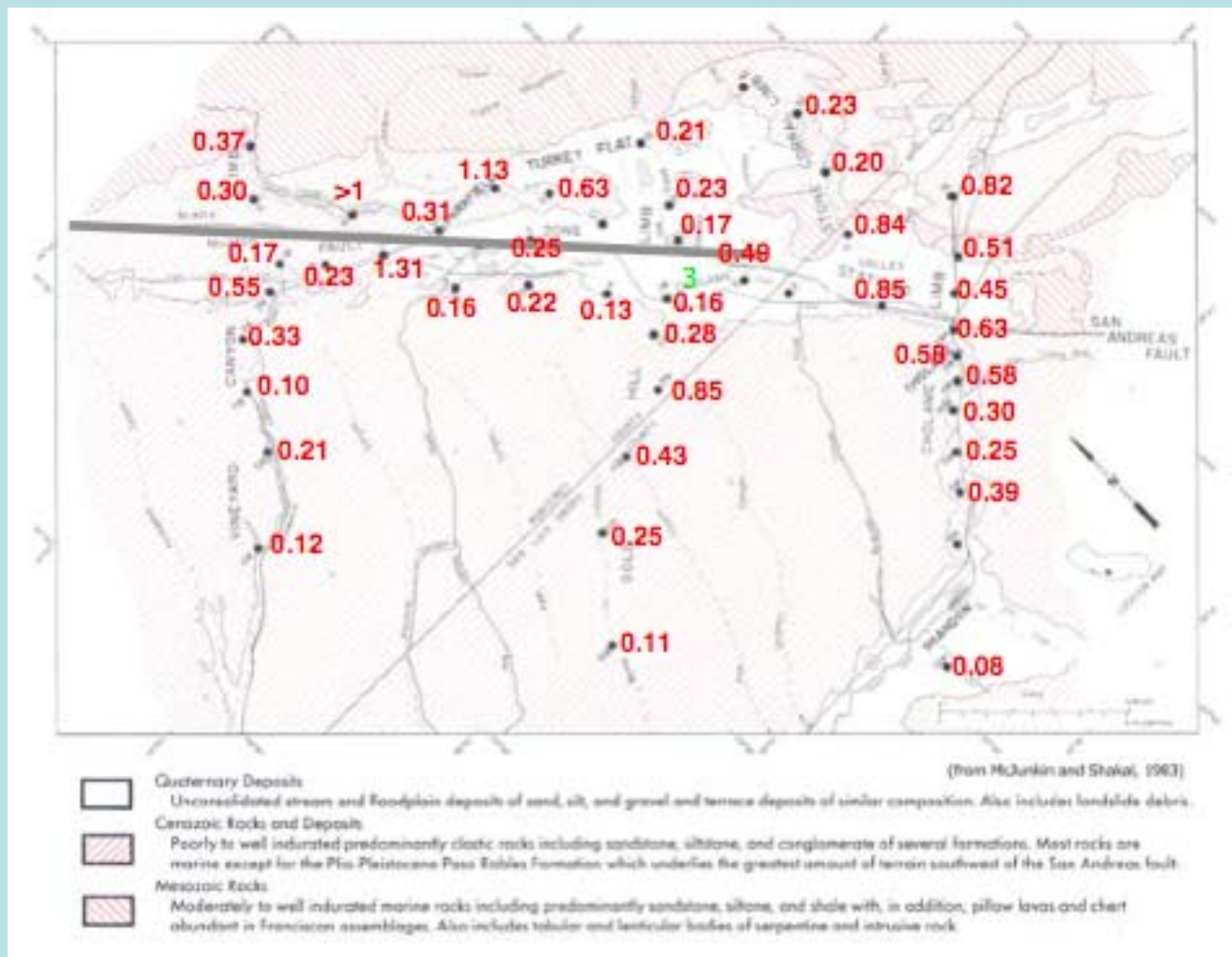
## M8, R10, SS, Rock, T=1 sec



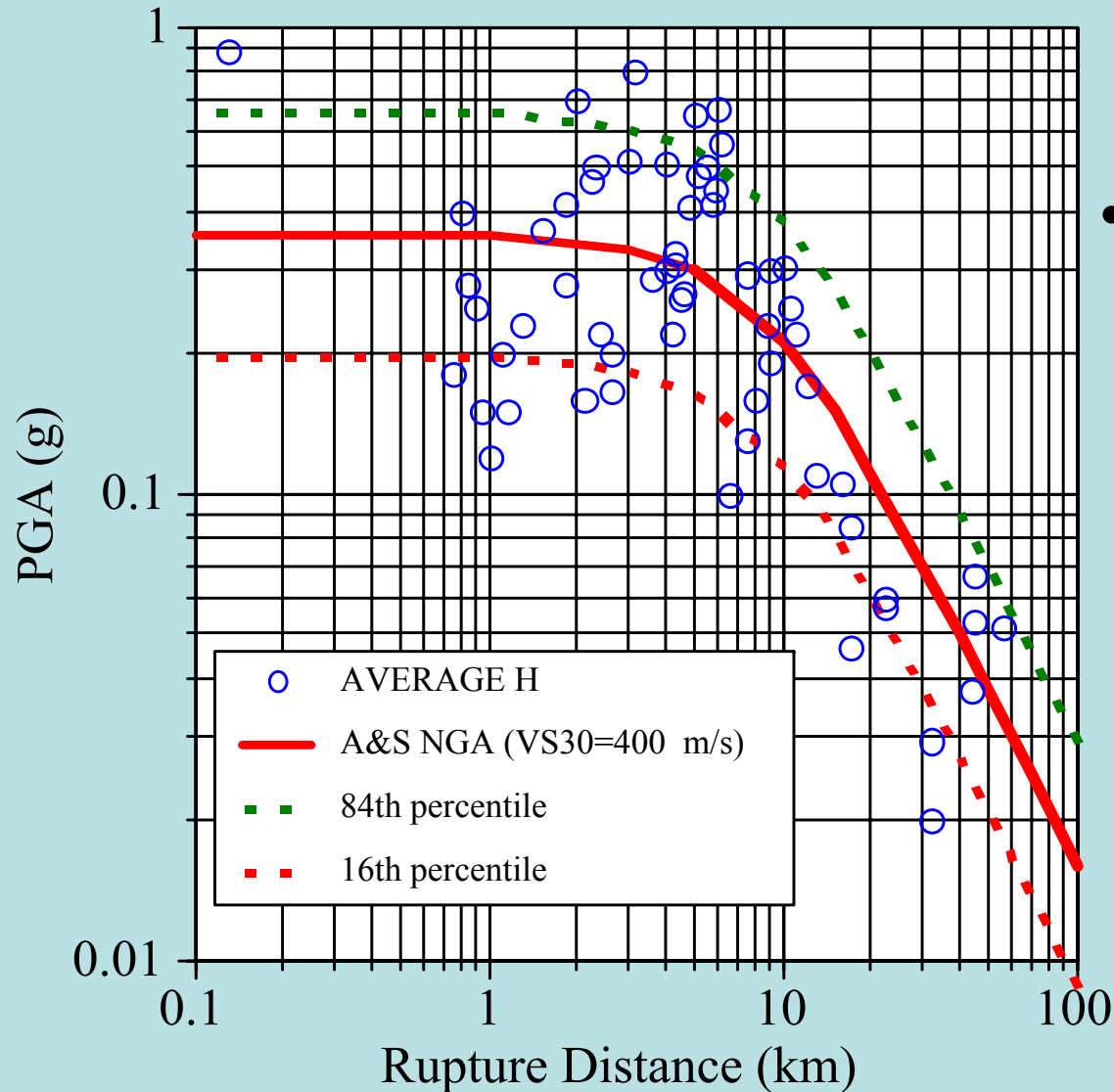
- NGA models have larger variability for M8
- This offsets some of the effect of reduction in median

# What about 2004 Parkfield (M6)?

- Very high ground motions observed
- Several  $PGA > 1g$
- Most are much smaller



# Parkfield PGA



- Parkfield PGA values are consistent with NGA models
  - Median
  - Standard deviation

# Characteristics of Rock Motions Including Near-Fault Effects

# Near Fault Effects

- Directivity
  - Related to the direction of the rupture front
    - Forward directivity: rupture toward the site (site away from the epicenter)
    - Backward directivity: rupture away from the site (site near the epicenter)
- Fling
  - Related to the permanent tectonic deformation at the site

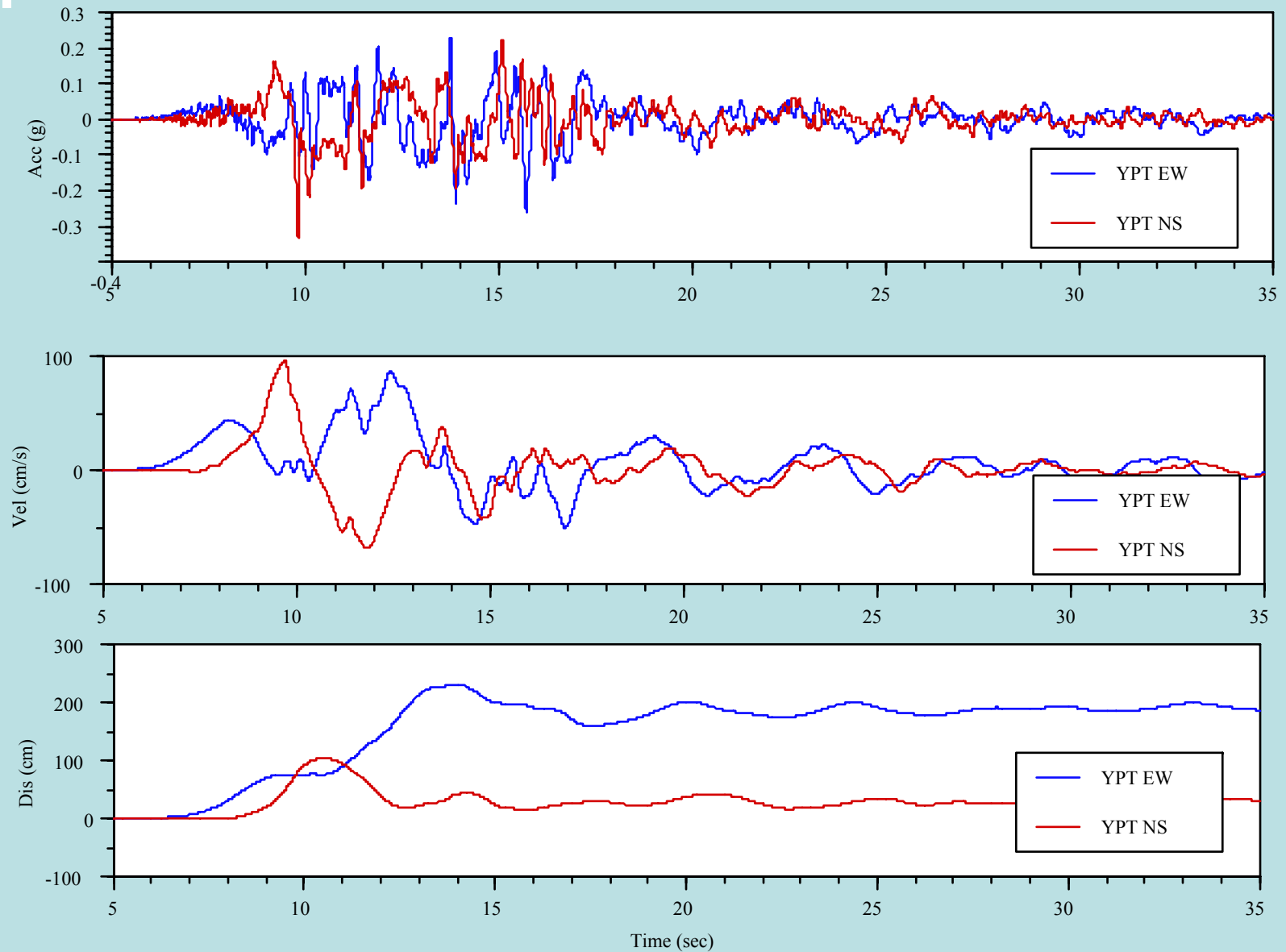
# Velocity Pulses

- Forward Directivity
  - Two-sided velocity pulse due to constructive interference of SH waves from generated from parts of the rupture located between the site and epicenter
    - Constructive interference occurs if slip direction is aligned with the rupture direction
    - Occurs at sites located close to the fault but away from the epicenter for strike-slip
- Fling
  - One-sided velocity pulse due to tectonic deformation
  - Occurs at sites located near the fault rupture independent of the epicenter location

# Observations of Directivity and Fling

<u>Sense of Slip</u>	<u>Directivity</u>	<u>Fling</u>
Strike-Slip	Fault Normal	Fault Parallel
Dip-Slip	Fault Normal	Fault Normal

# Example of Near-Fault Effects (Kocaeli Earthquake)

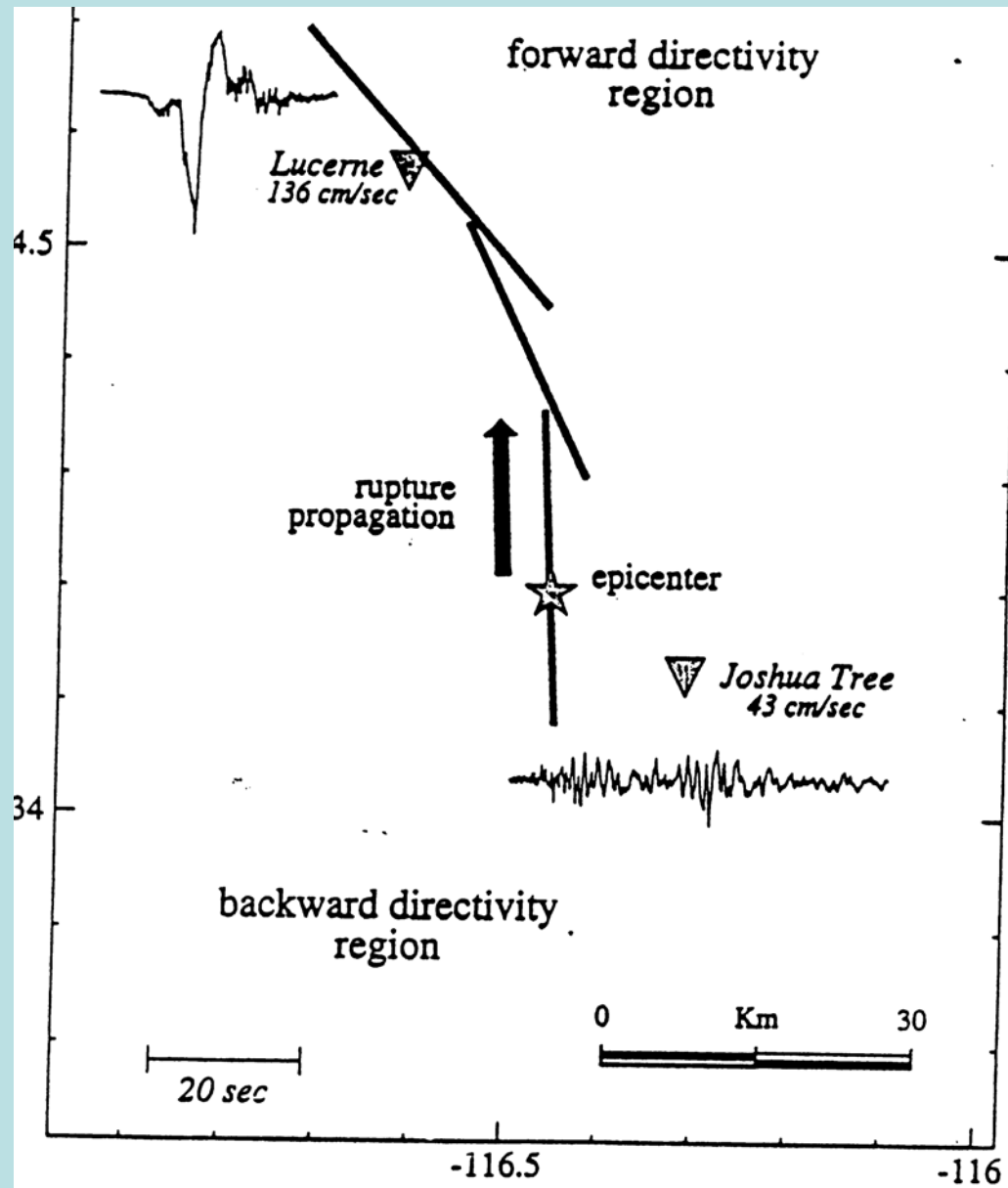


# Directivity Effects

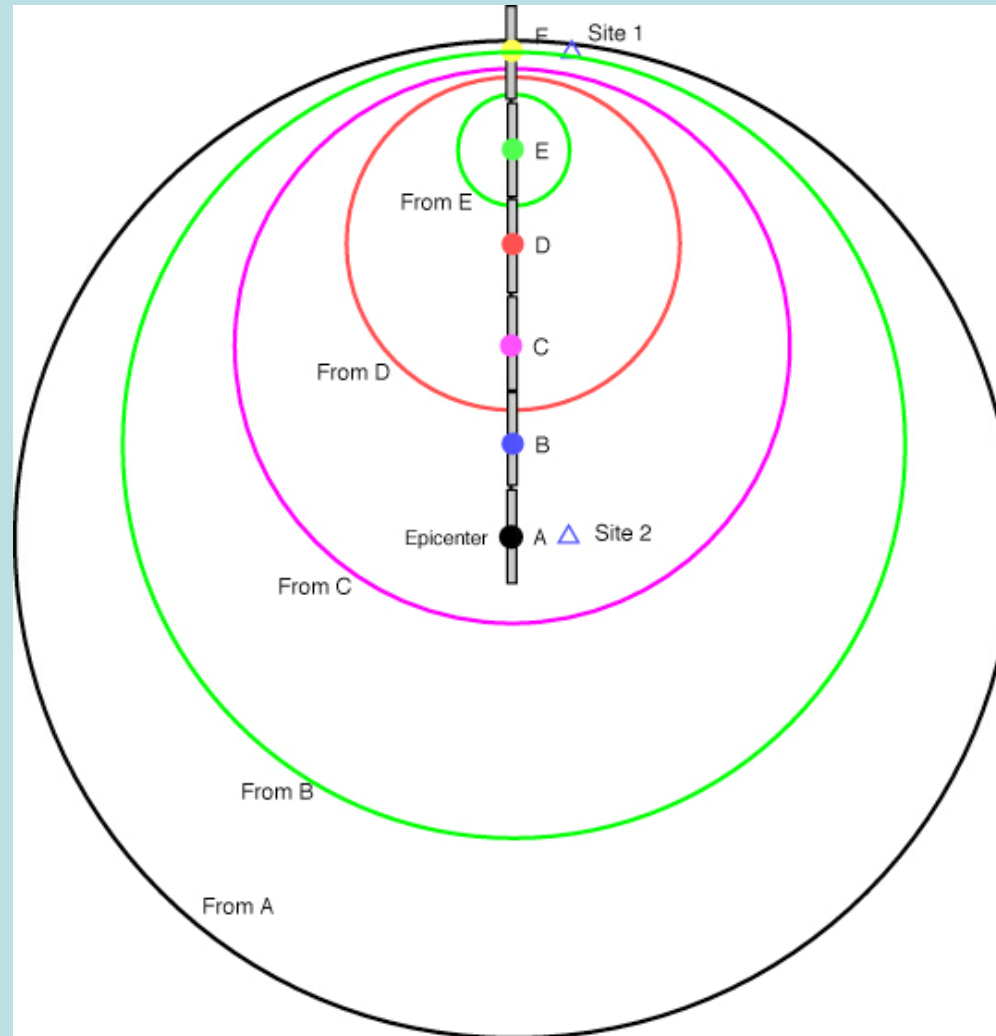
(Somerville et al, 1997)

## Two Effects on Ground Motion Amplitudes

- Changes in the average horizontal component as compared to standard attenuation relations
  - Increase in the amplitude of long period ground motion for rupture toward the site
  - Decrease in the amplitude of long period ground motion for rupture away from the site
- Systematic differences in the ground motions on the two horizontal components
  - Fault normal component is larger than the fault parallel component at long periods



# Directivity



# Model for Directivity Effects

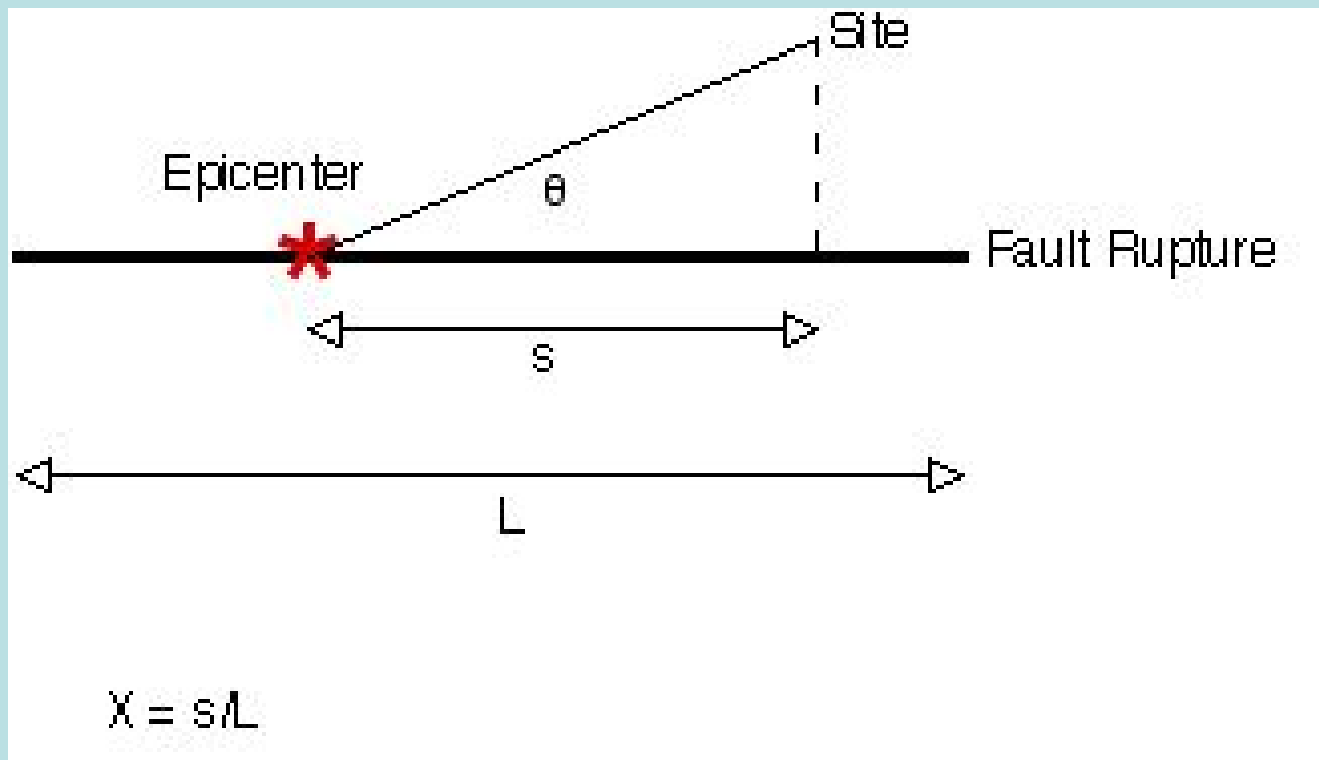
## Additional Parameters Required

- Strike-Slip Fault

$X$  = fraction of fault rupture between the epicenter and the site

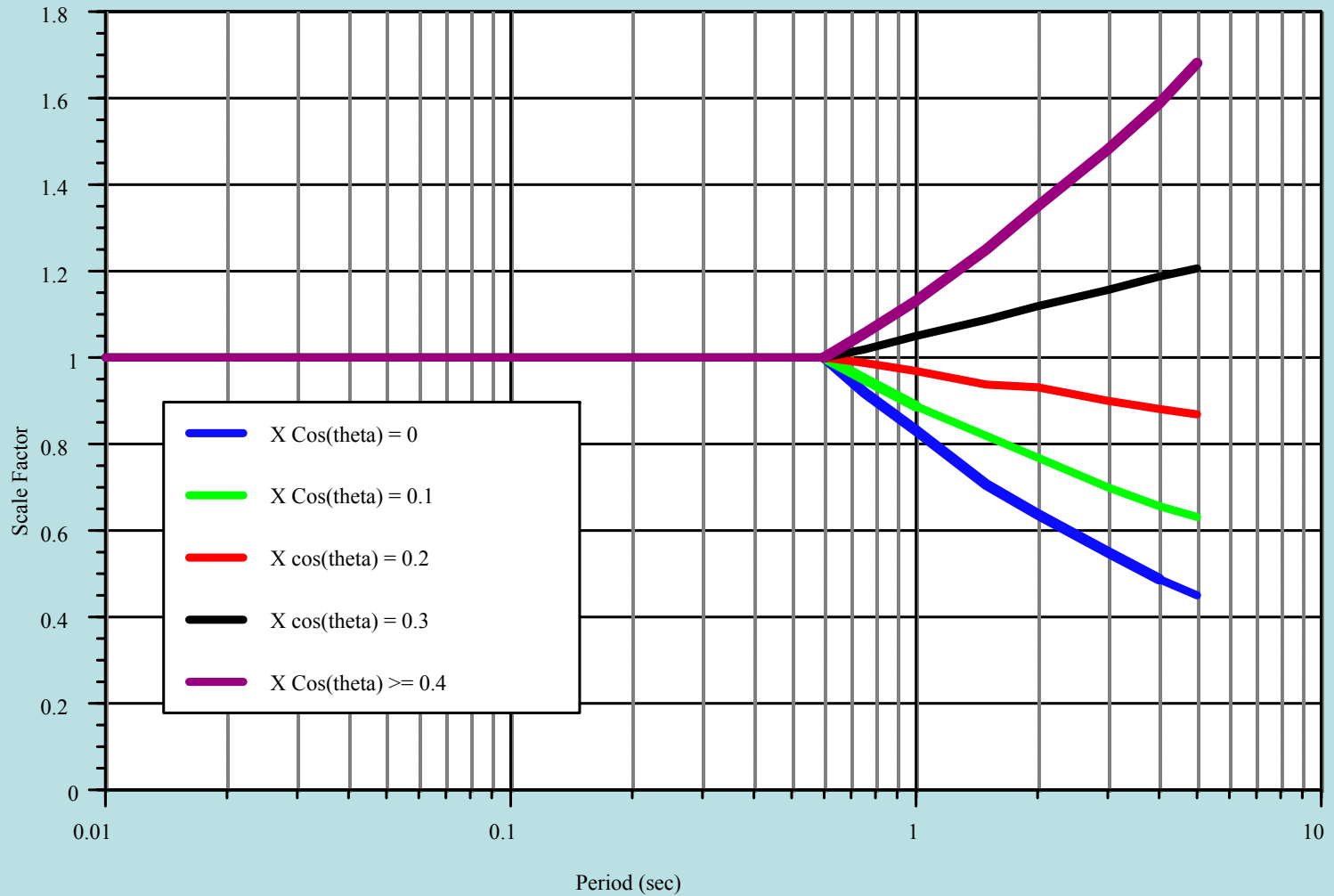
$\theta$  = angle between the fault strike and the epicentral direction from the site

# Directivity Parameters for Strike-Slip Faults

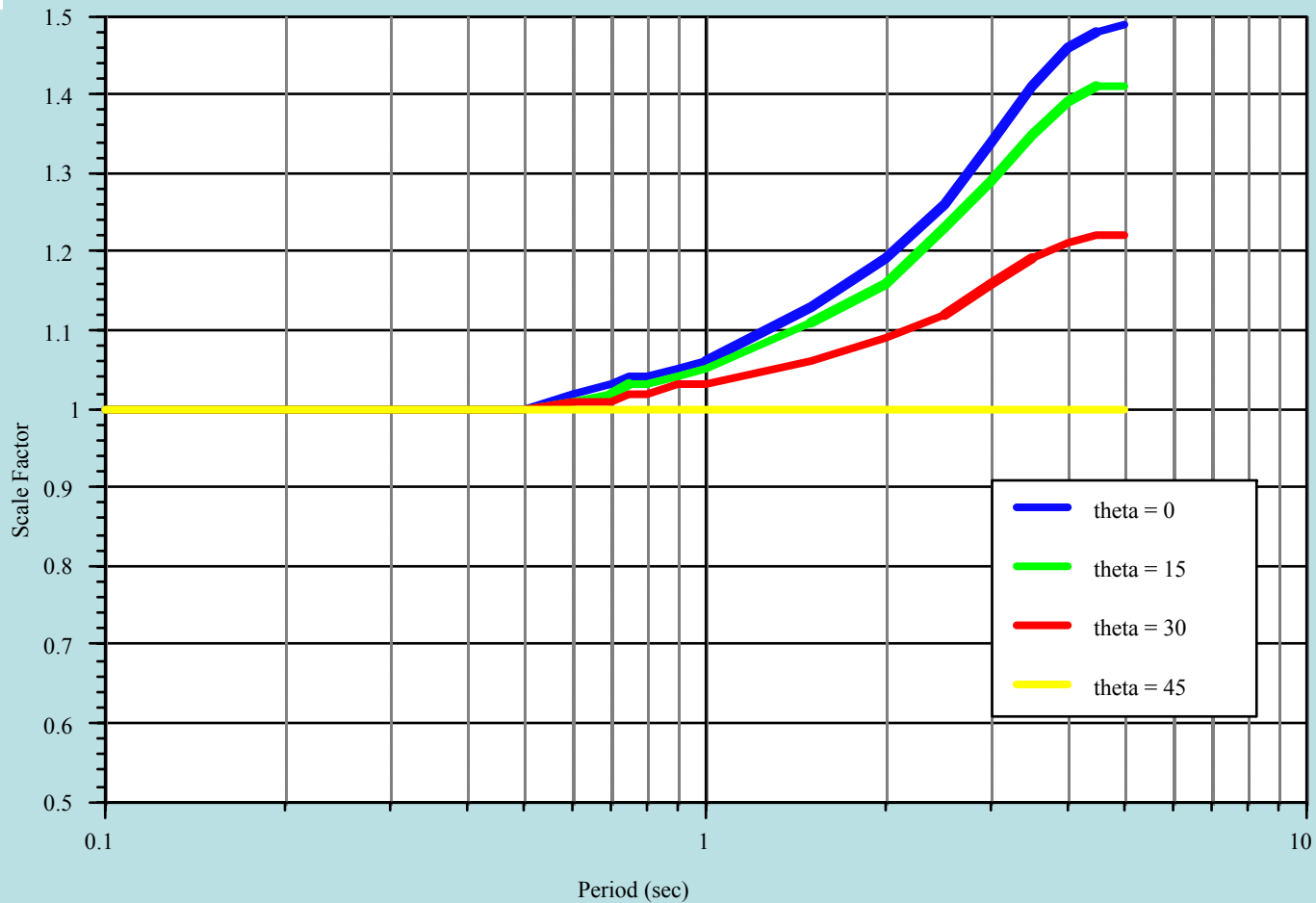


# Abrahamson (2000) Directivity Factors

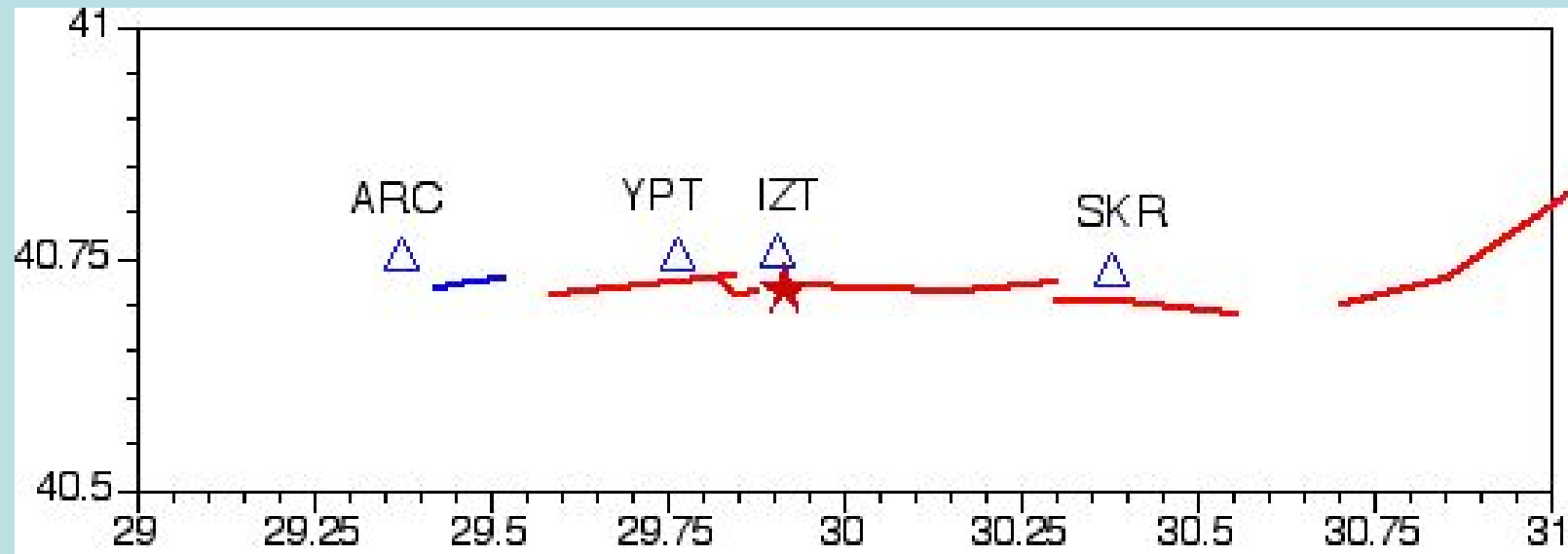
5% damping, Ave Horiz, Strike-Slip



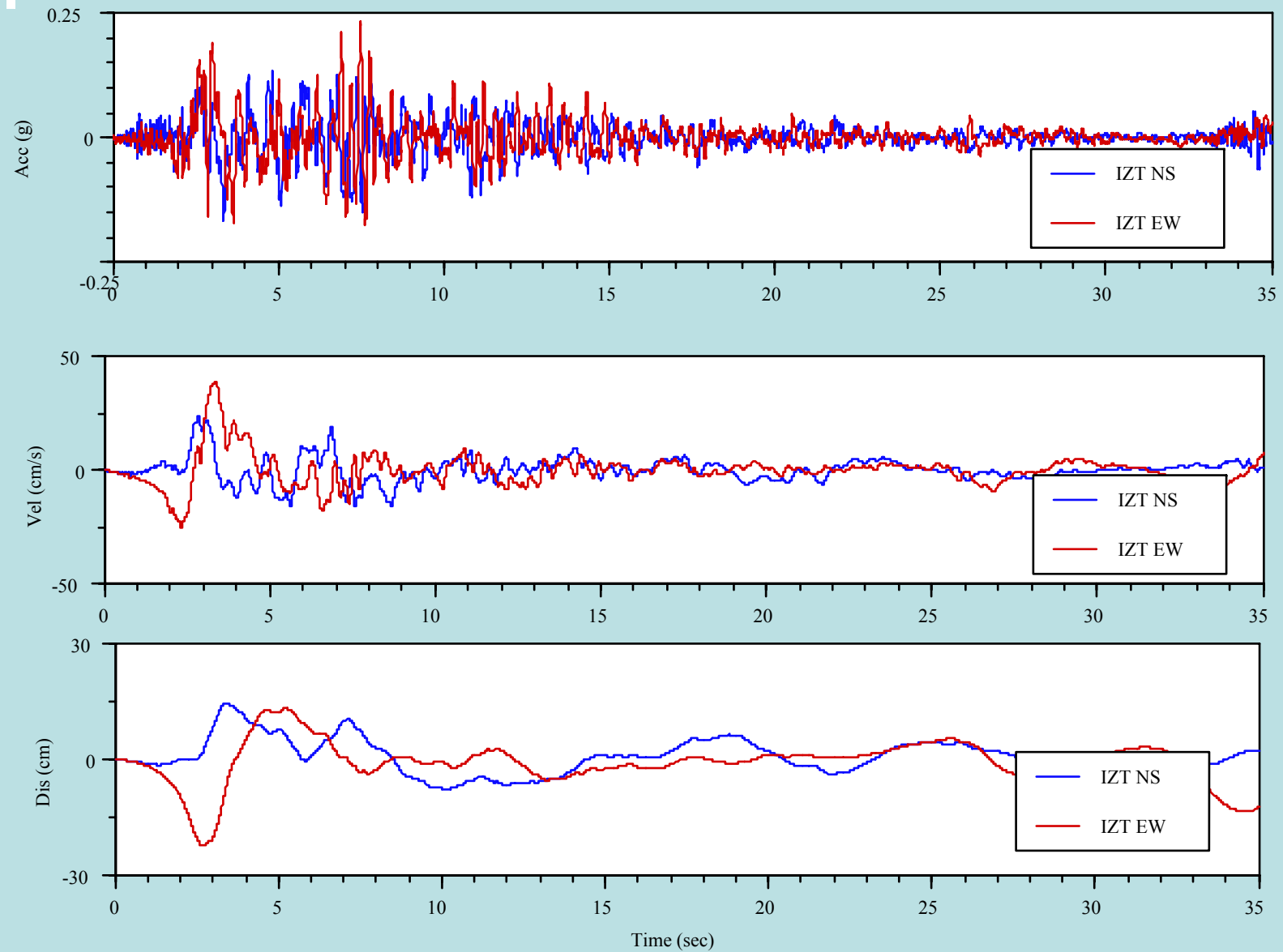
# Somerville et al (1997) Scale Factors for FN/Ave Horiz



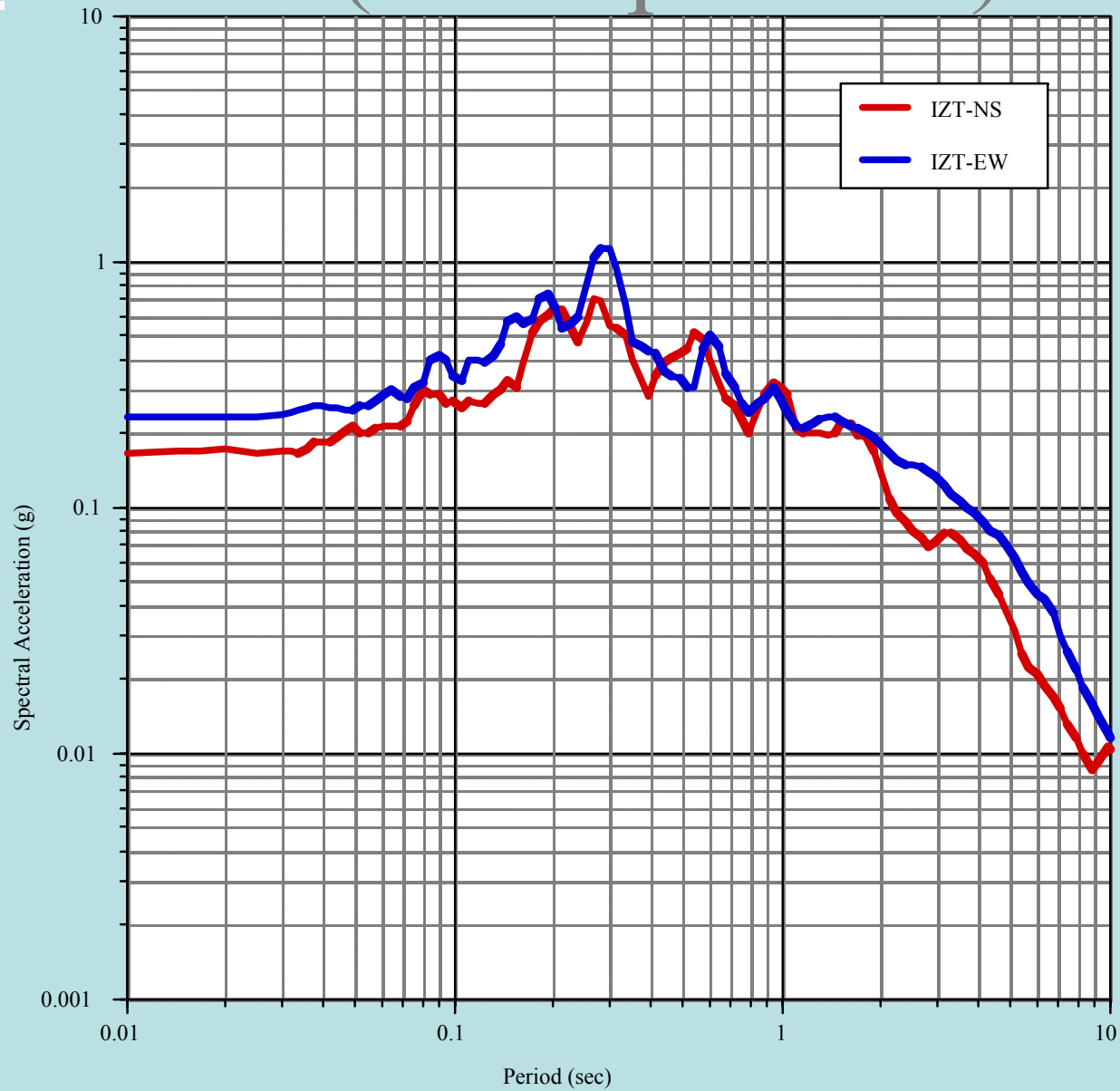
# Kocaeli Rupture and Strong Motion Stations



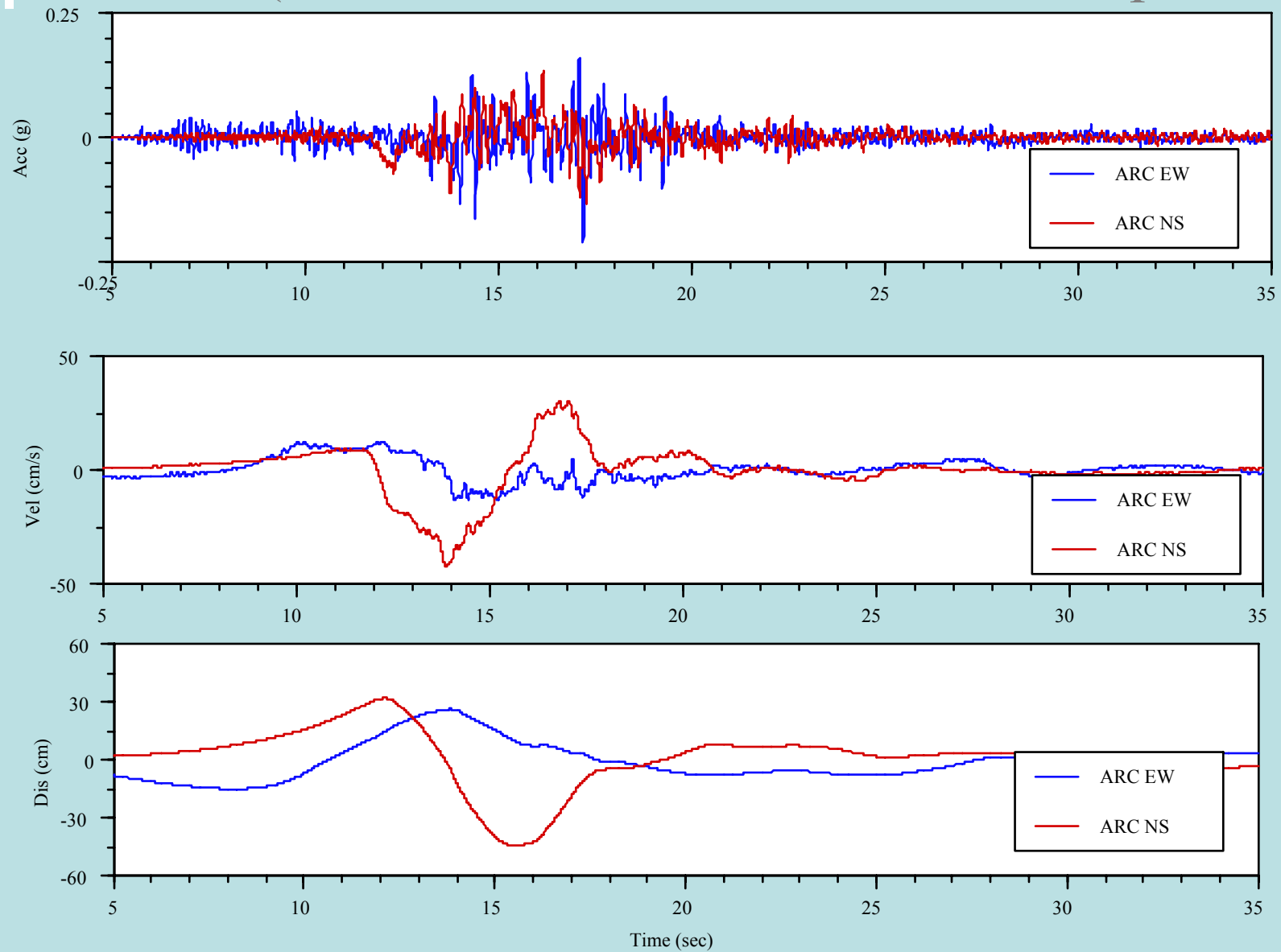
# IZT (near epicenter)



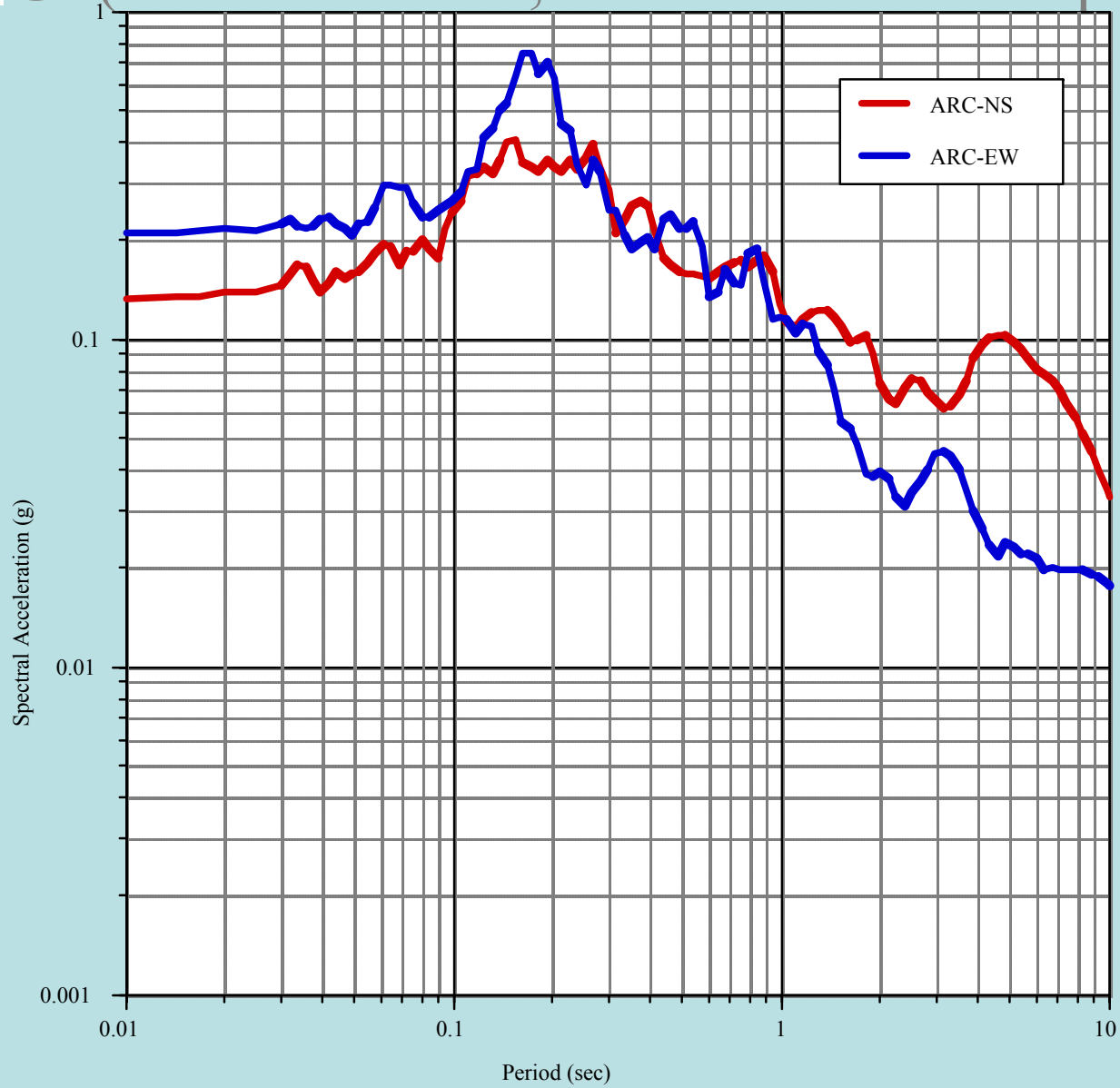
# IZT (near epicenter)



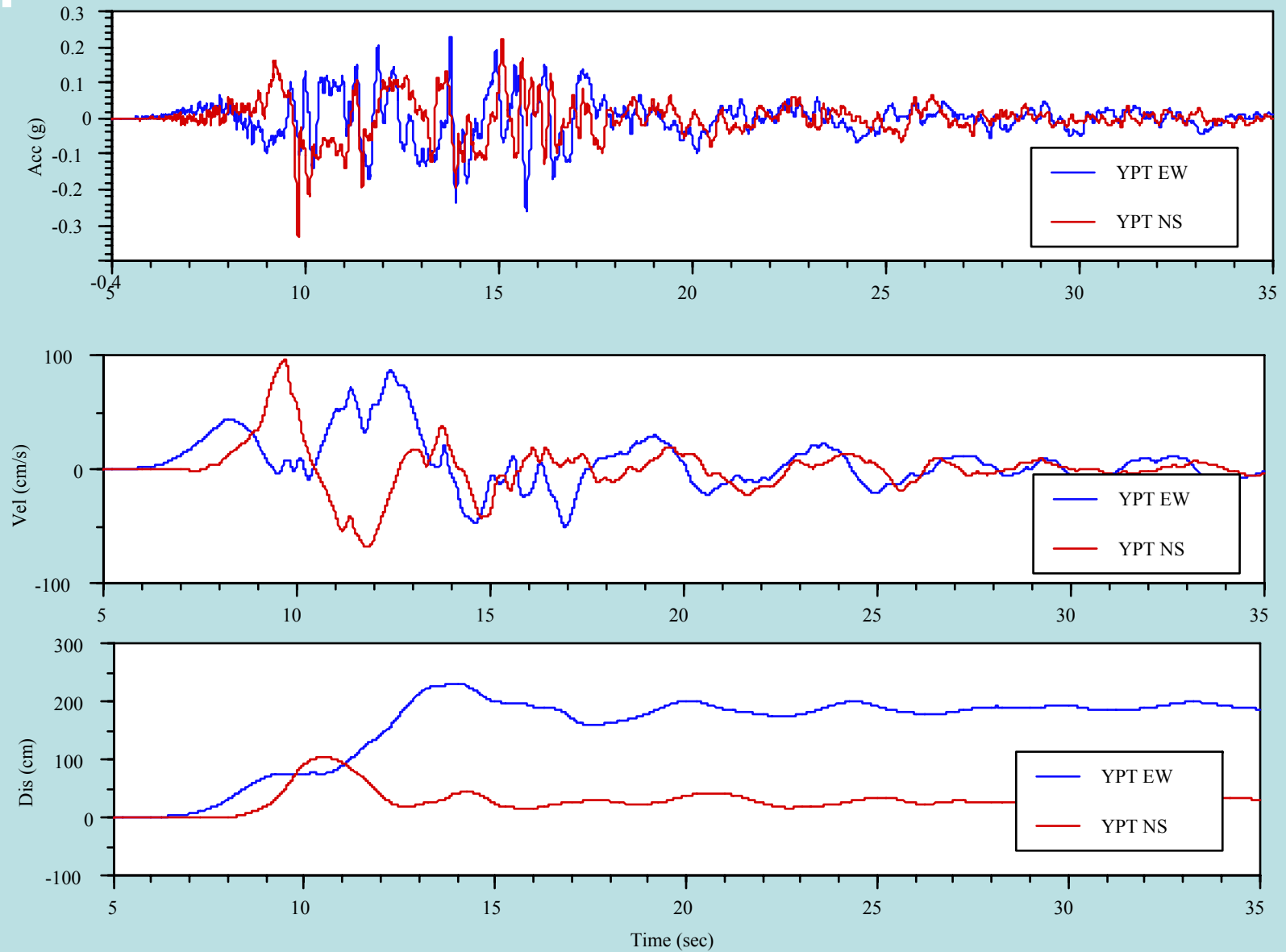
# ARC (off end of fault, down strike from epicenter)



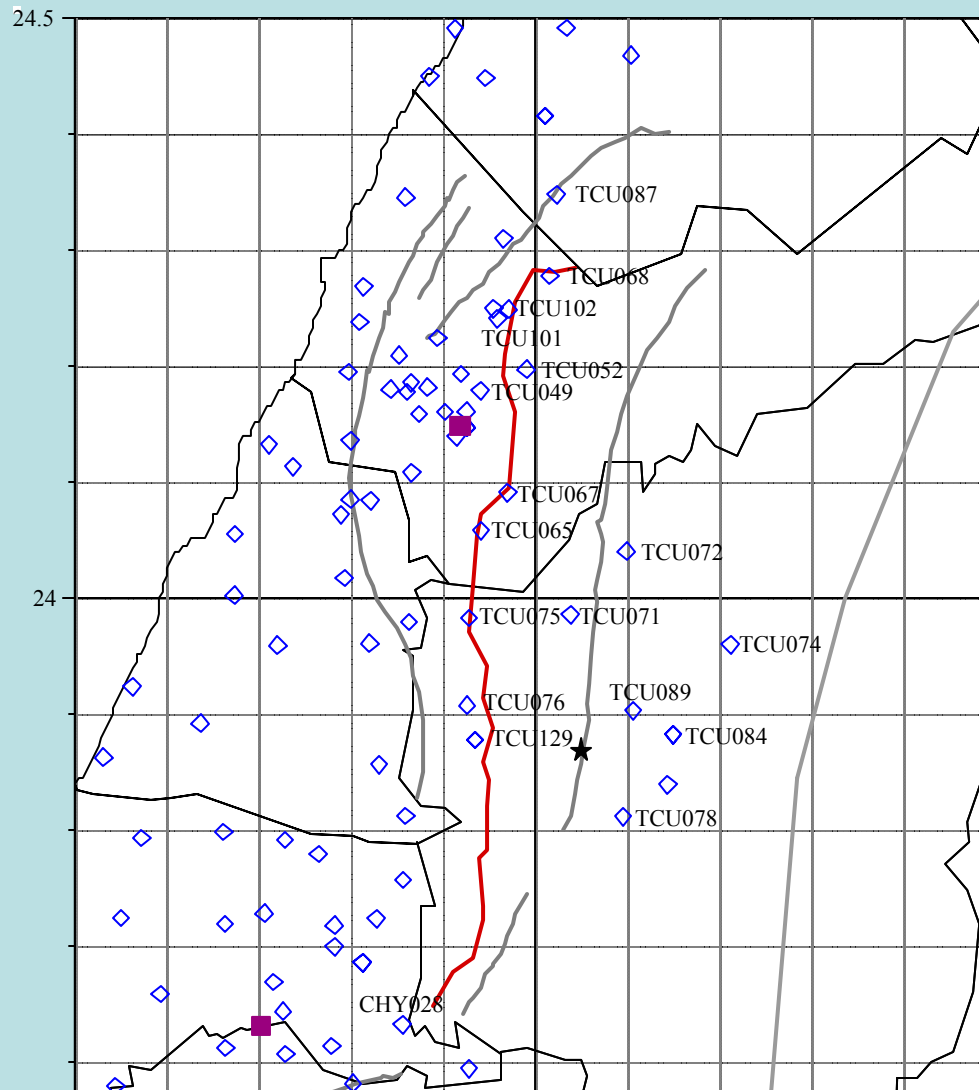
# ARC (off end of fault, down strike from epicenter)



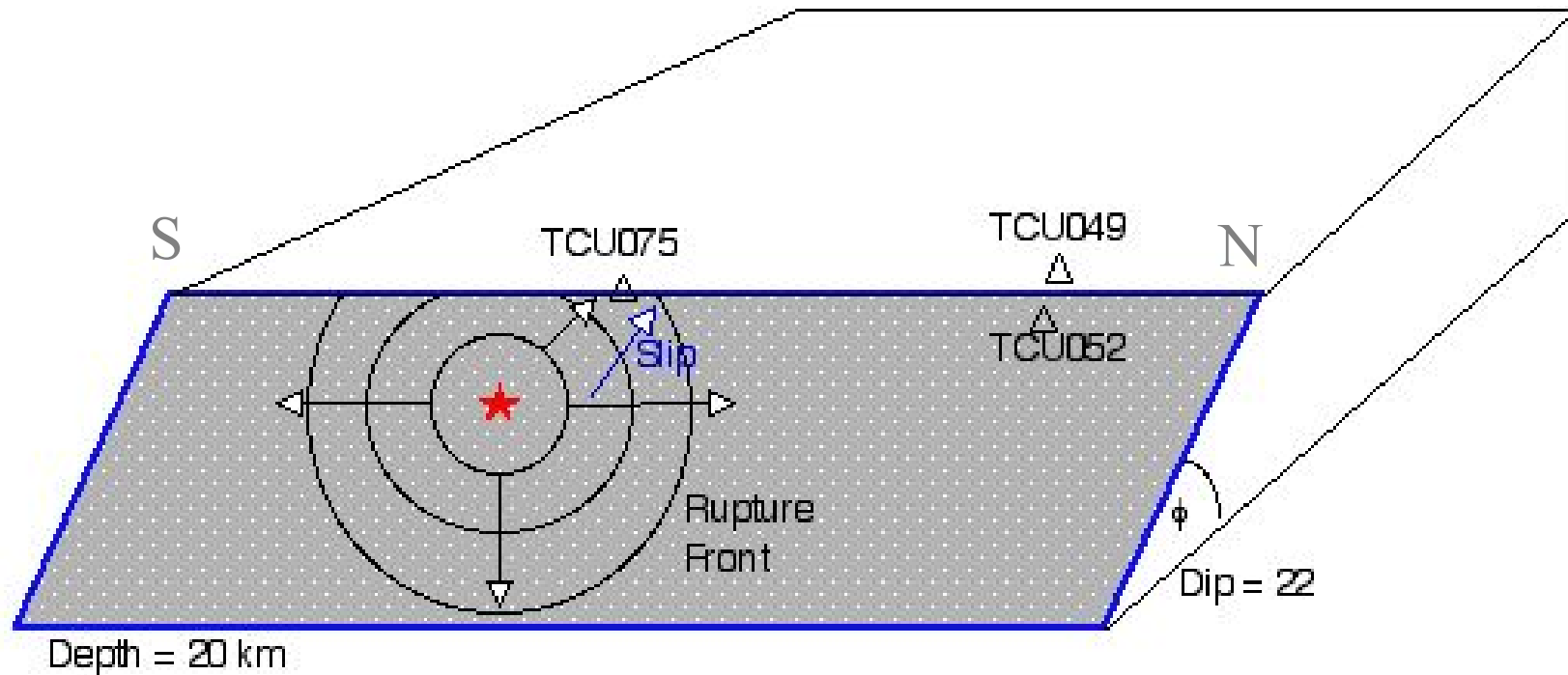
# YPT (near fault, down strike from epicenter)



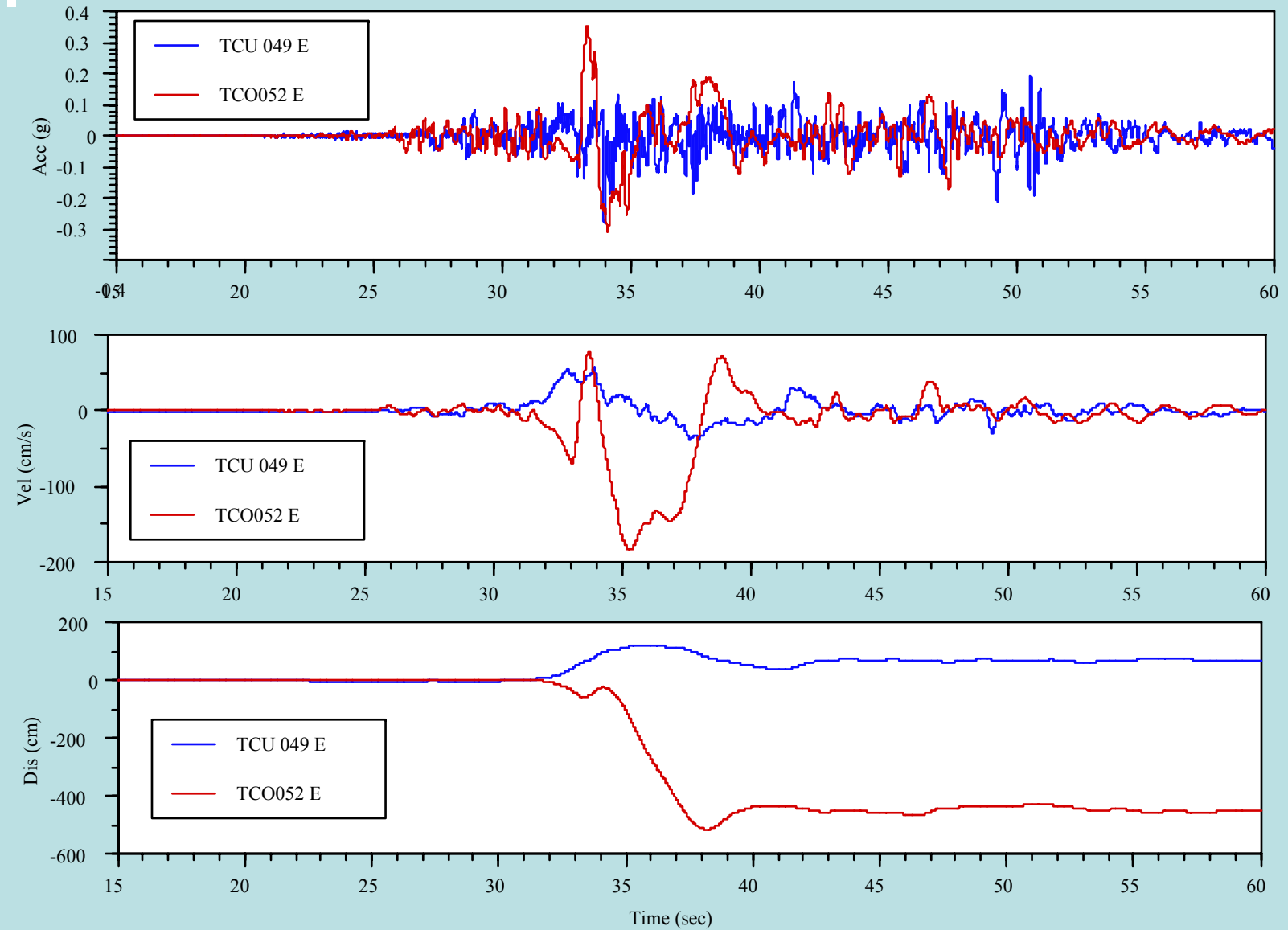
# Strong Motion Stations from the Chi-Chi Earthquake



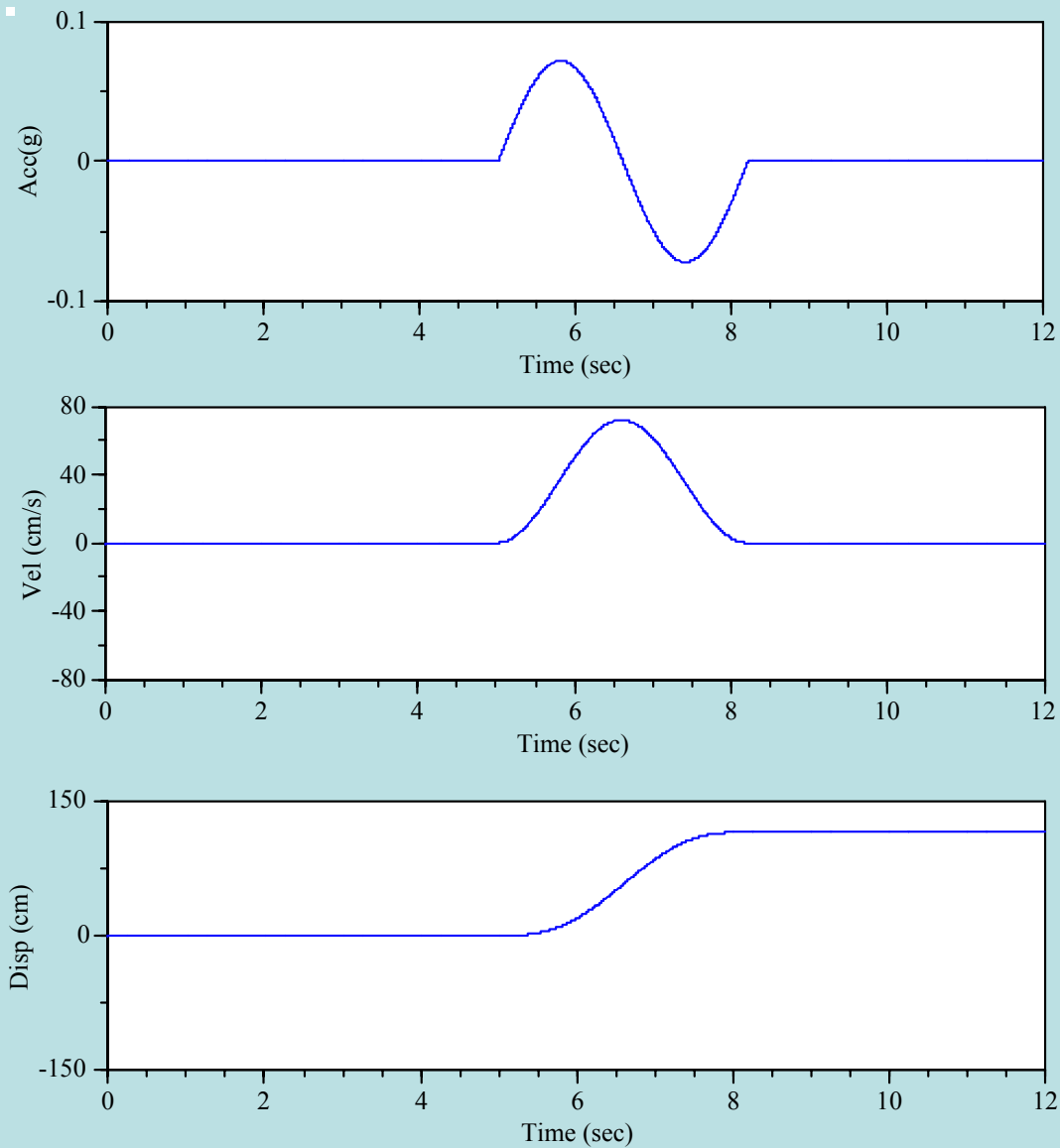
# Chi-Chi Earthquake



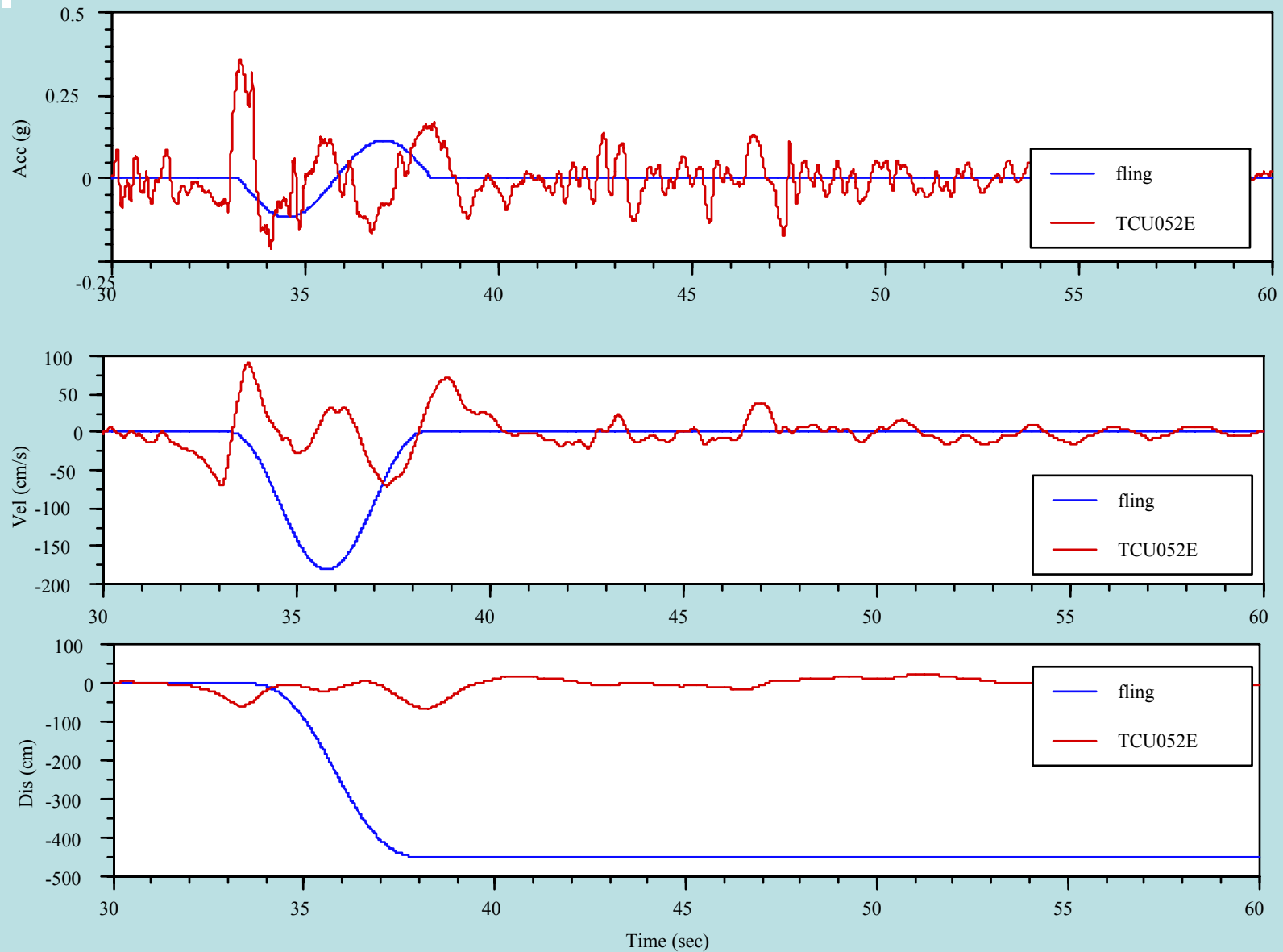
# Fling Effects



# Time Domain Fling Model



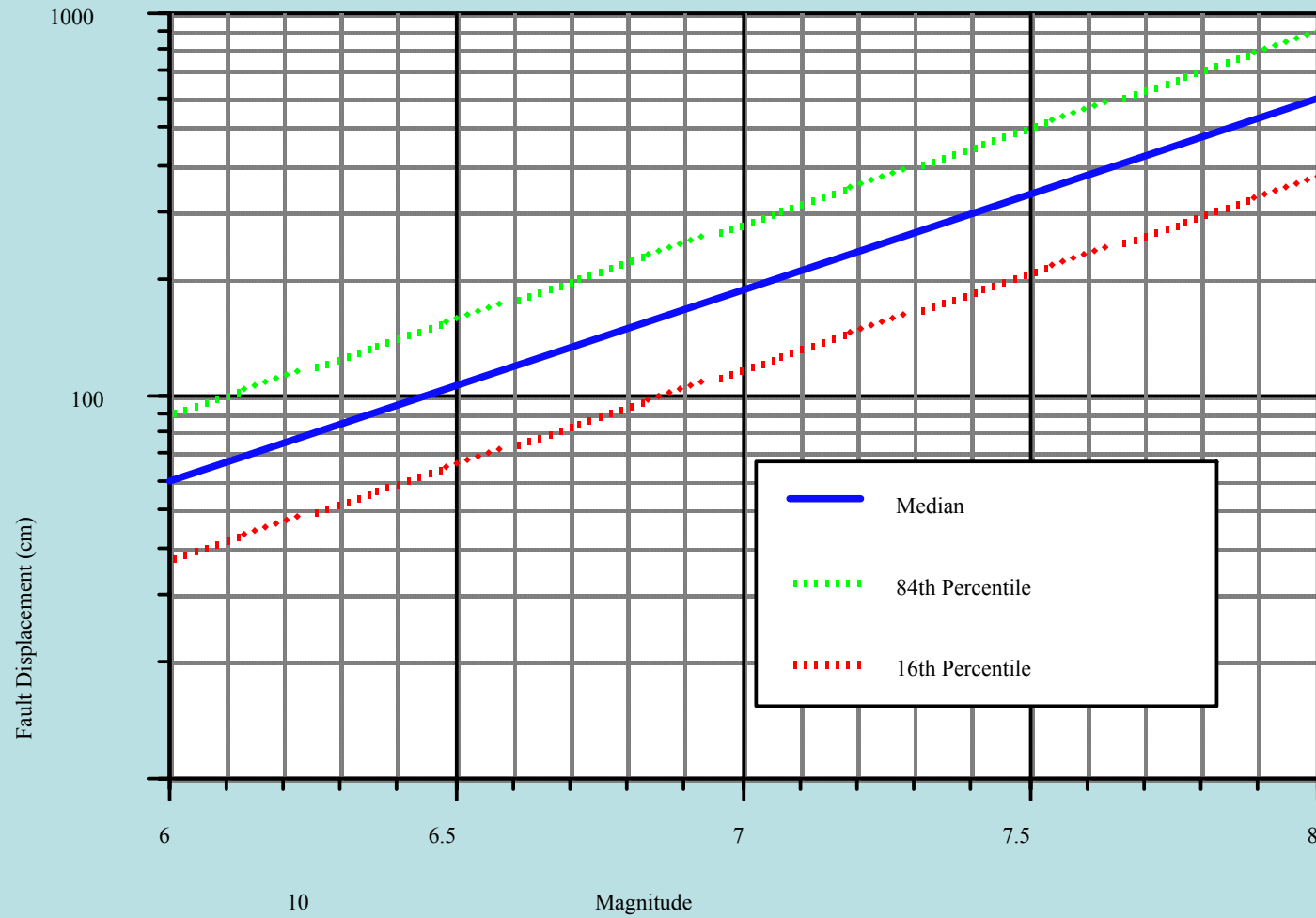
# Separation of Fling and Wave Propagation Effects



# Parameters Required for Fling

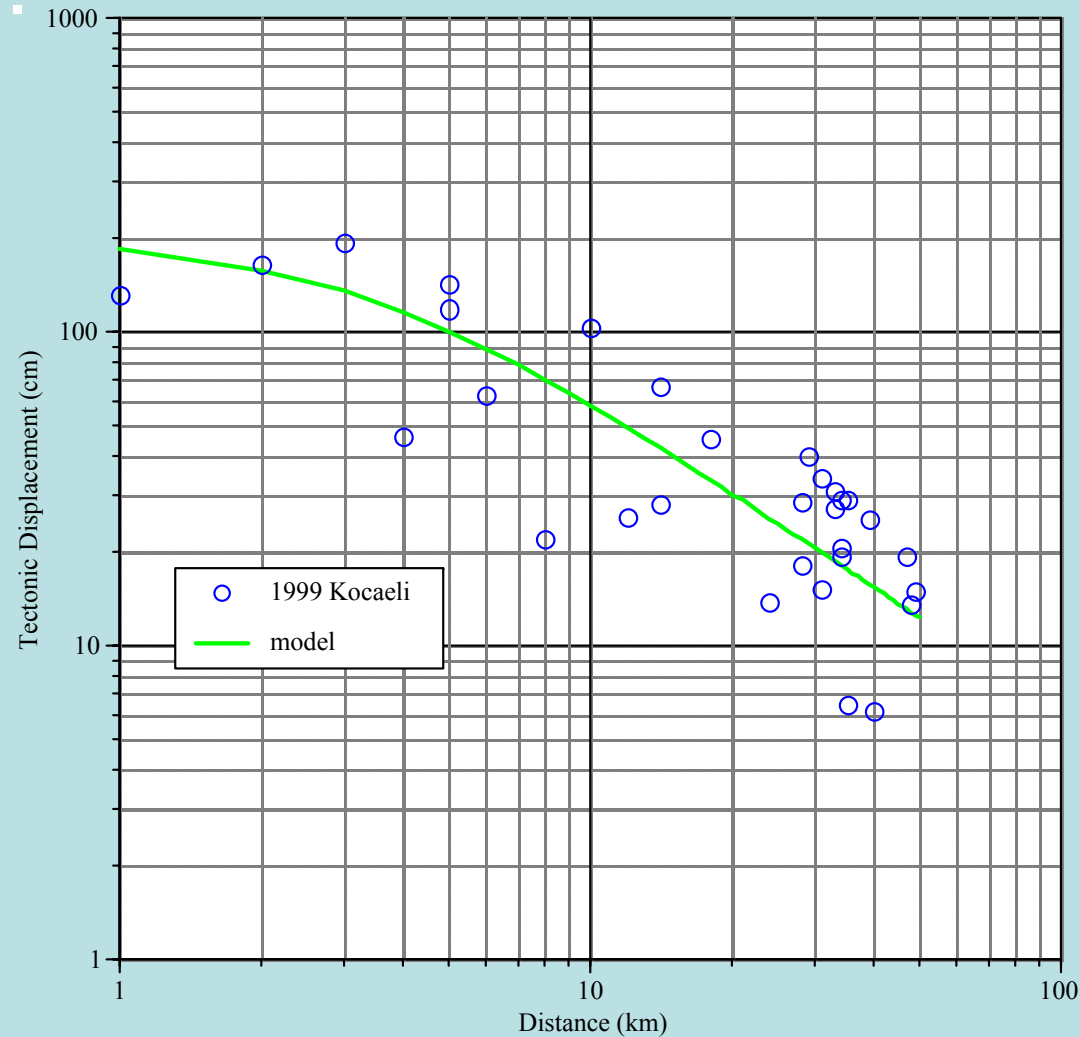
- Amplitude of Fling
  - From fault slip and geodetic data
- Duration (period) of Fling
  - From strong motion data
- Arrival Time of Fling
  - From numerical modeling
  - Relative timing of fling and S-waves

# Fault Displacement

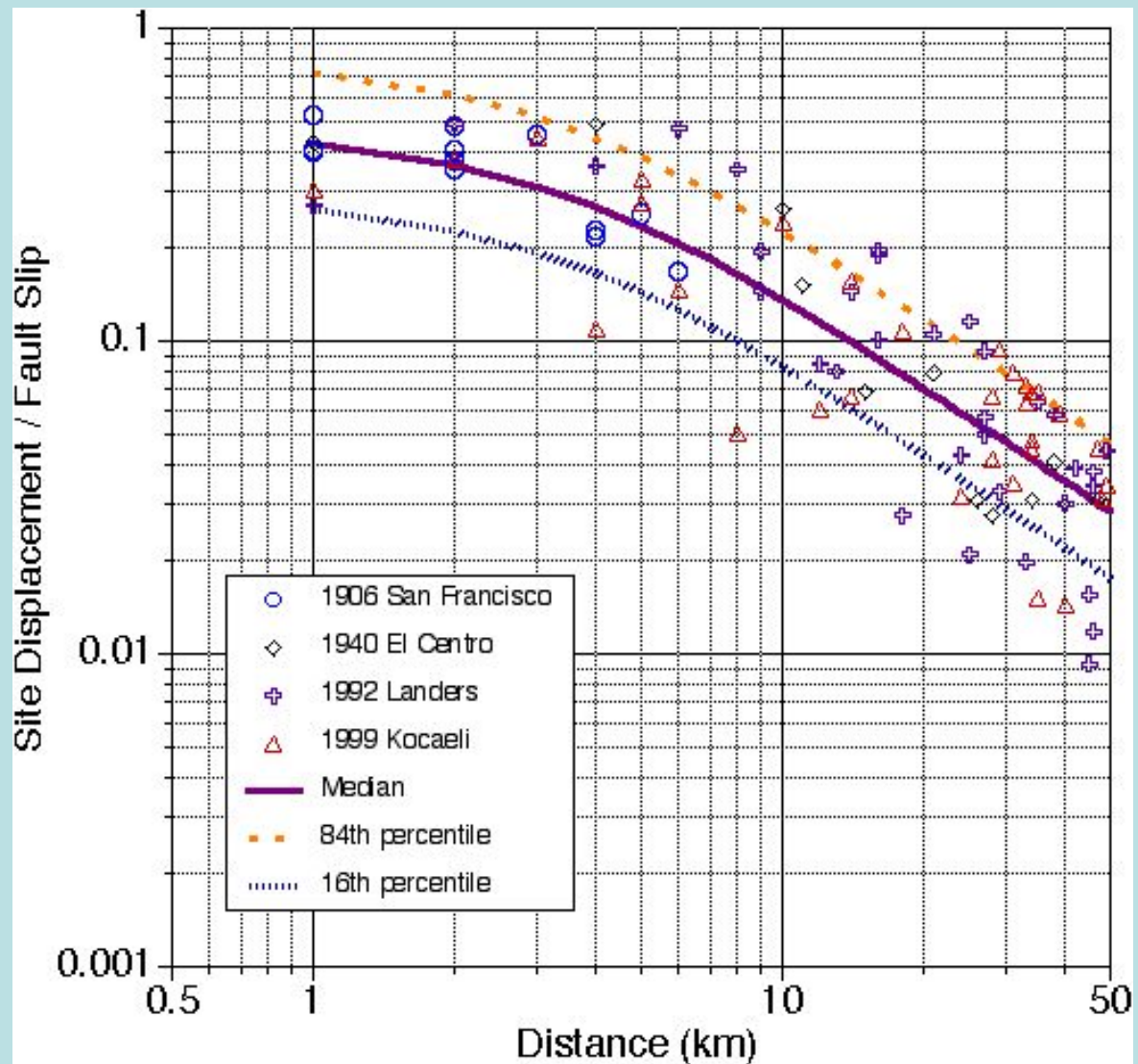


# Attenuation of Fling Amplitude

## Example from Kocaeli Geodetic Data

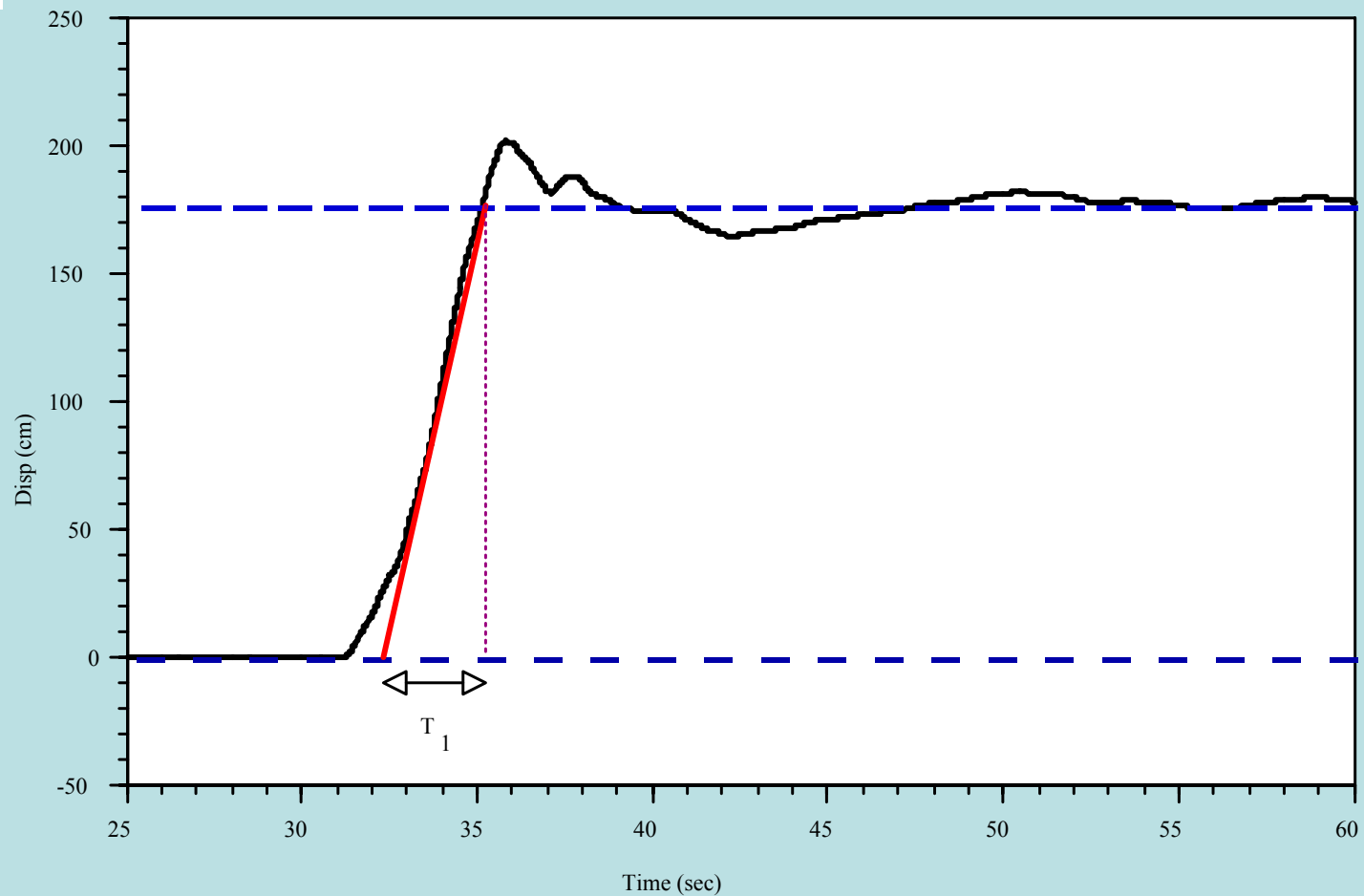


# Attenuation of Fling Amplitude

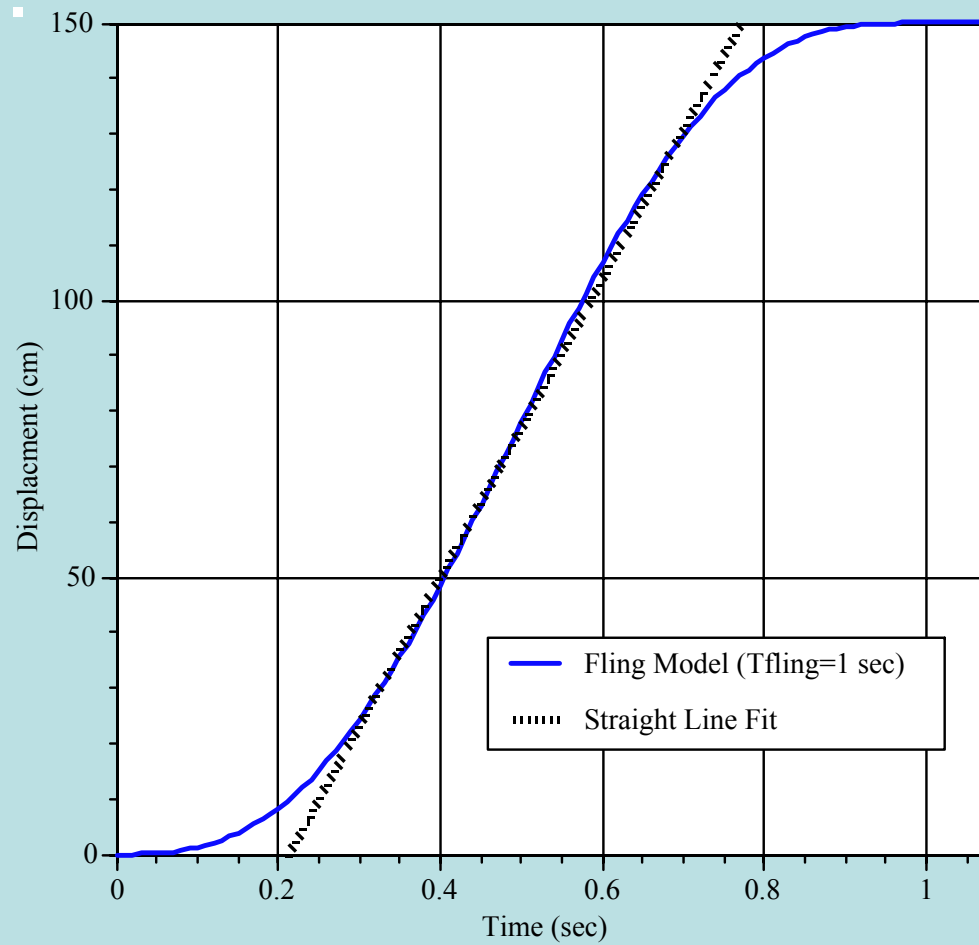


# Duration of Fling

Measured from Strong Motion Recordings  
(SKR from Kocaeli)

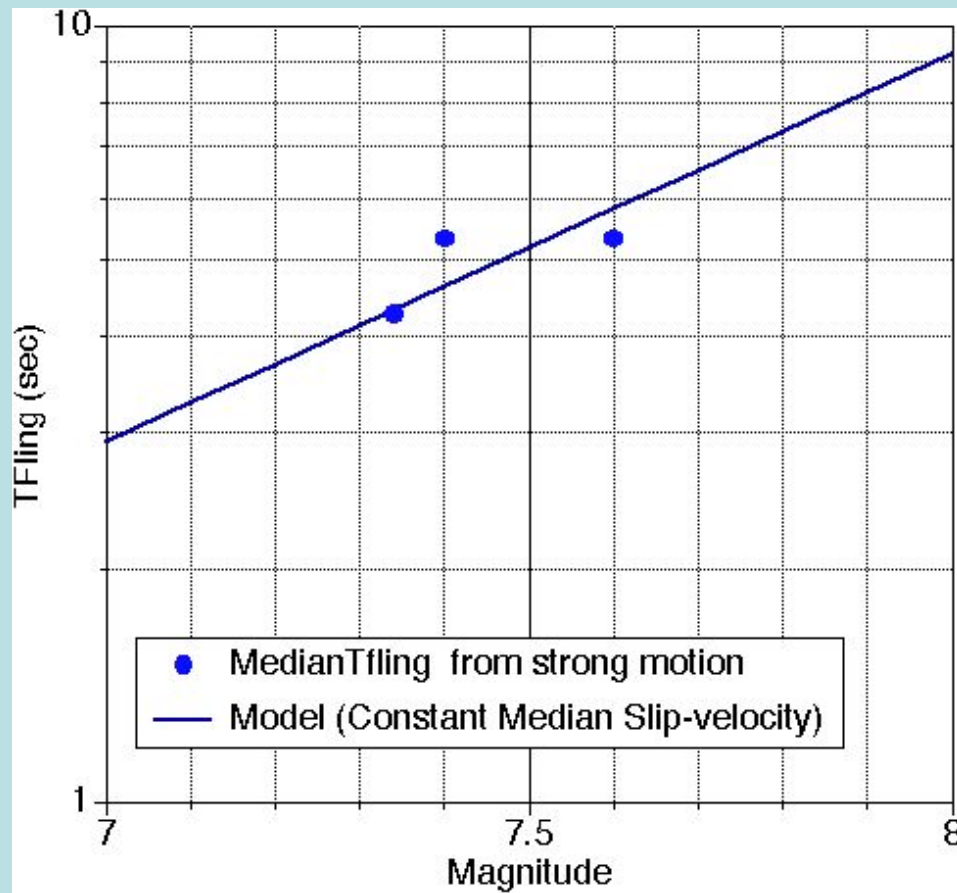


# Fling Period



# Model for Duration of Fling

(slope fixed by assuming median slip-velocity is independent of magnitude)



# Summary of Near Fault Effects from Recent Earthquakes

- Near fault ground motions can have large velocity pulses caused by directivity and/or fling
- Forward Directivity Effects
  - Observed in Kocaeli and Denali earthquakes
    - Consistent with previously derived models
  - Not observed in Chi-Chi earthquake due to shallow depth of hypocenter
- Fling
  - Observed in Kocaeli, Chi-Chi, and Denali

# Near Fault Effects

- Current scaling relations for directivity effects are consistent with data from new earthquakes
- Current attenuation relations do not include fling
  - Fling effects scale differently with magnitude and distance than ground motion due to wave propagation
- A separate ground motion model is needed for the fling, which then needed to be combined with the ground motion due to wave propagation

# Example Application: DCPD

- Design Earthquake
  - $M=7.25$
  - Distance = 4.5 km
  - Strike-Slip Mechanism

# Directivity Parameters

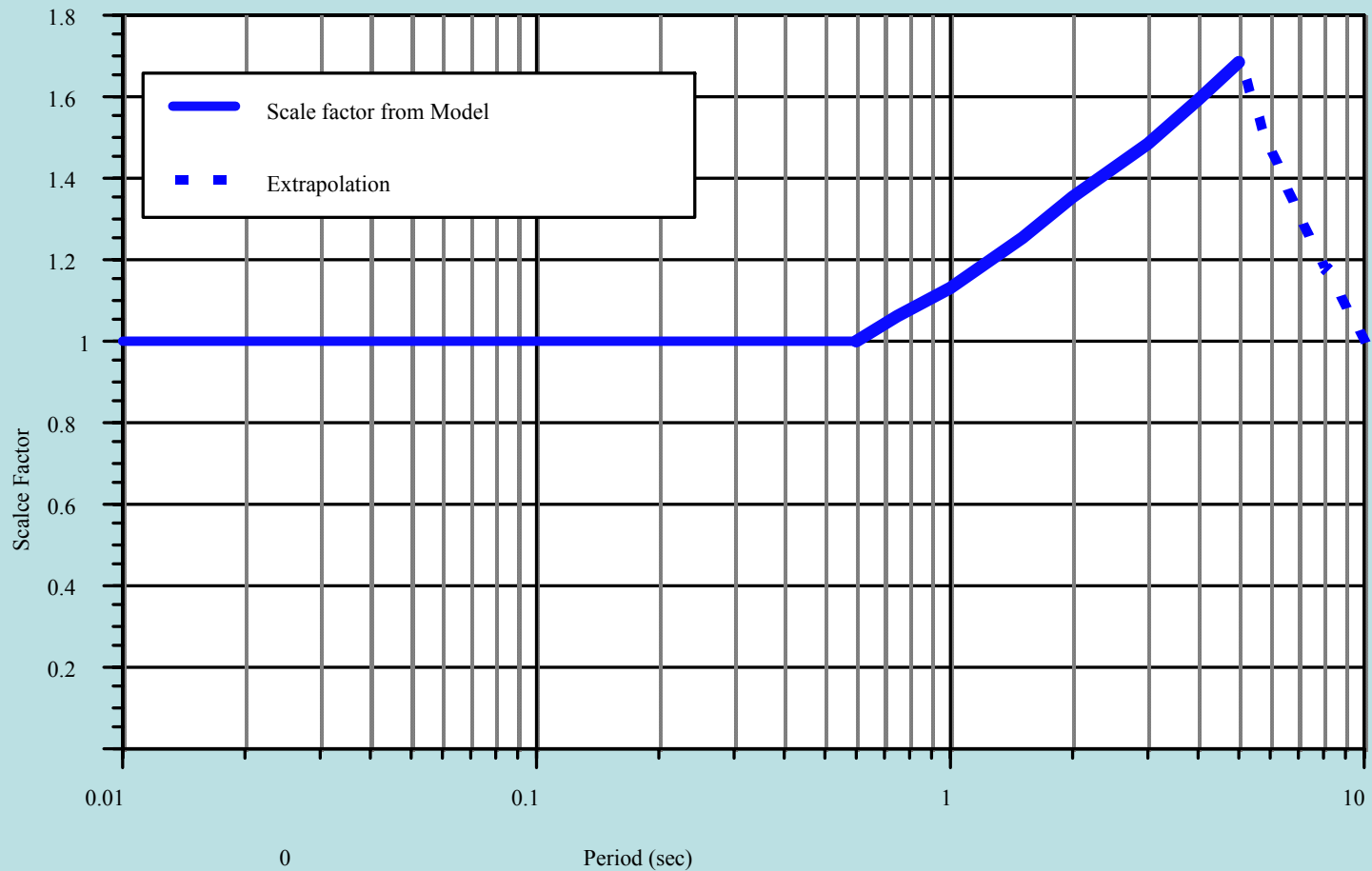
- Epicenter assumed to be located at end of Hosgri fault (maximum directivity effect)

$$X = 70 \text{ km} / 110 \text{ km} = 0.64$$

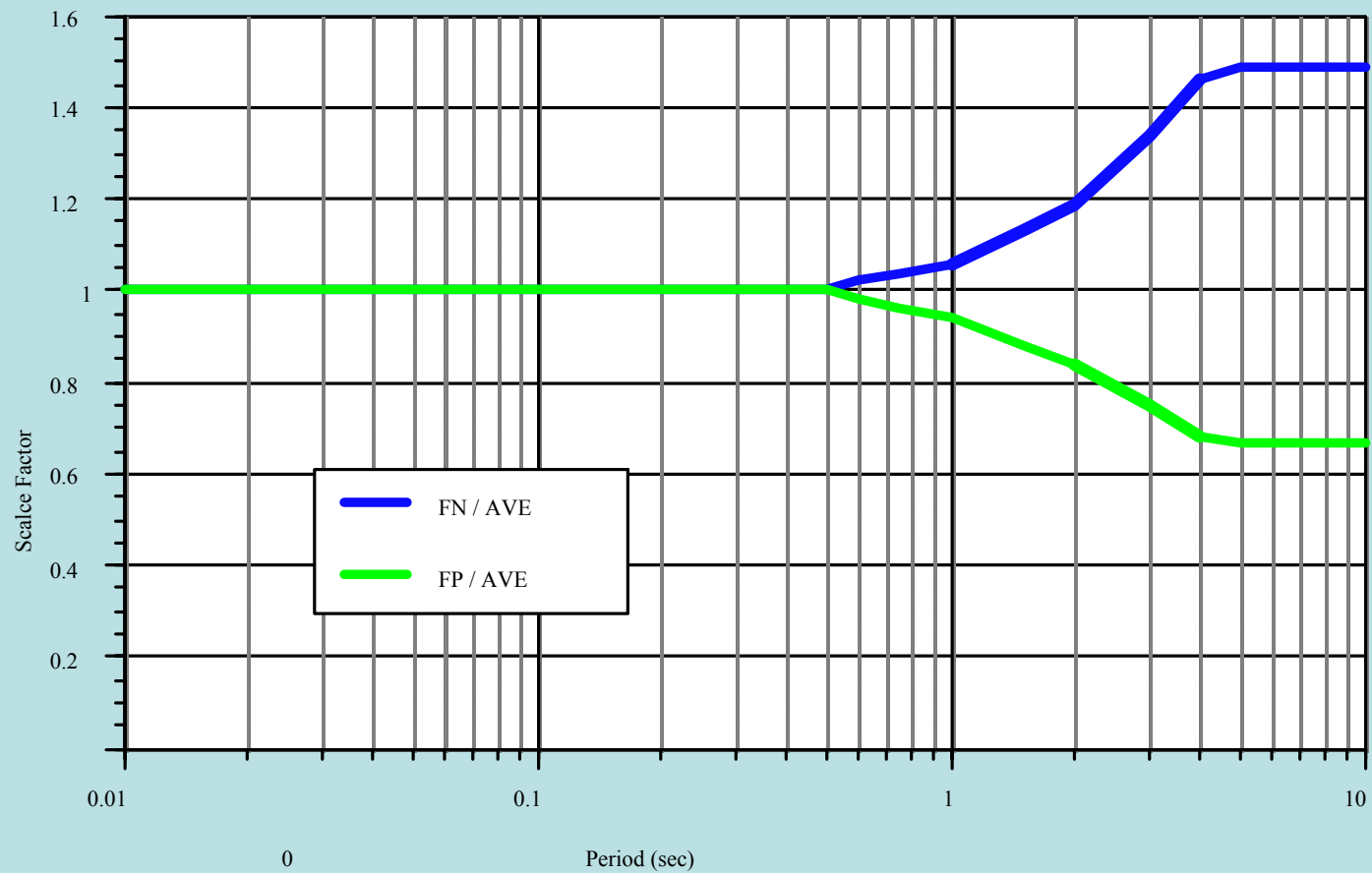
$$\theta = 3 \text{ degrees}$$

$$X \cos(\theta) = 0.64$$

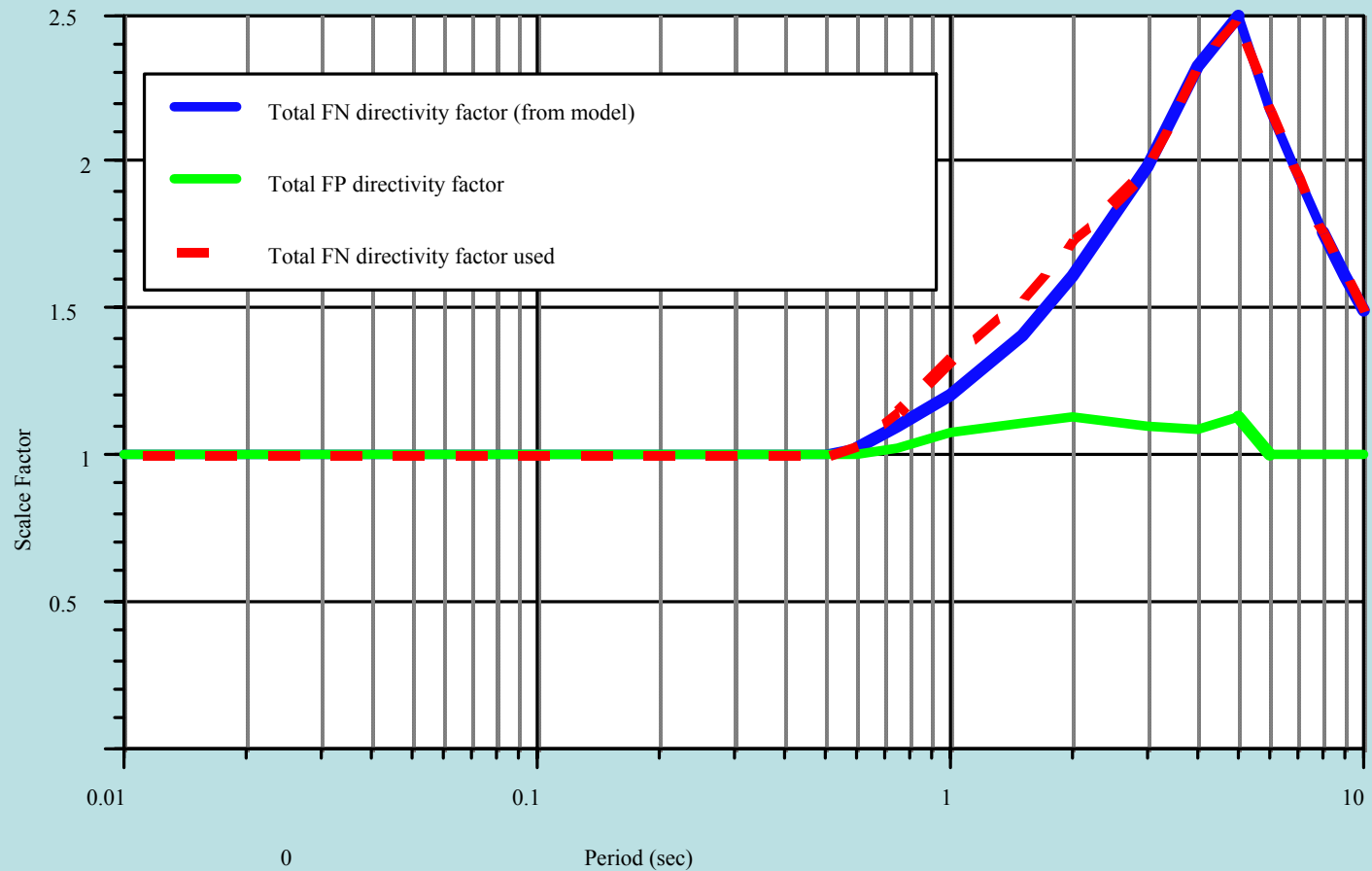
# Directivity Effects on the Average Horizontal Component



# Directivity Effects on FN/Ave and FP/Ave



# Combined Directivity Effects



# Fling

- Use 84th Percentile
  - Two parameters: Displacement at site, Fling period
    - Use 84th percentile displacement,
    - Use fling period to give 84th percentile acceleration

## Fling Displacement

Median slip on fault = 233 cm

Median disp at site = 59 cm

84th percentile disp at site = 115 cm

## Fling Period

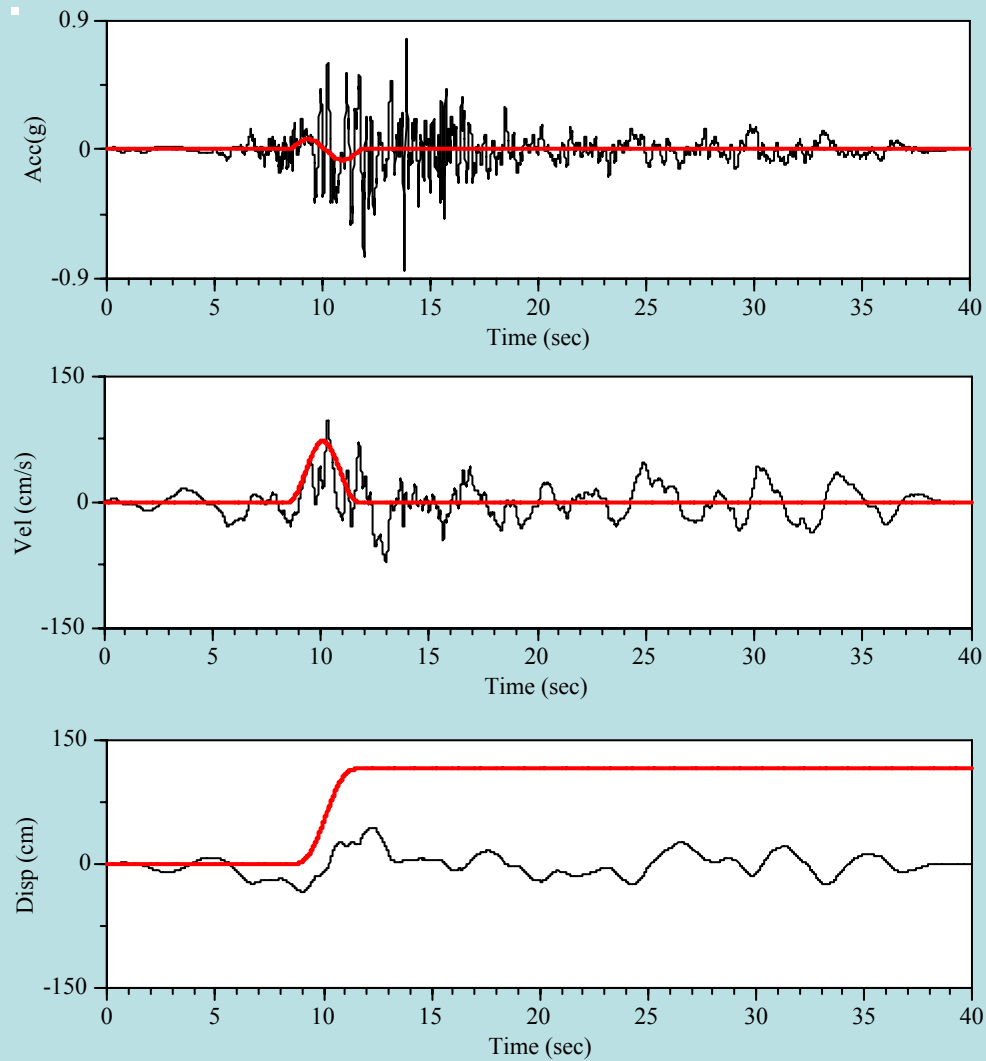
3.2 sec

(84th percentile acc = 0.072g)

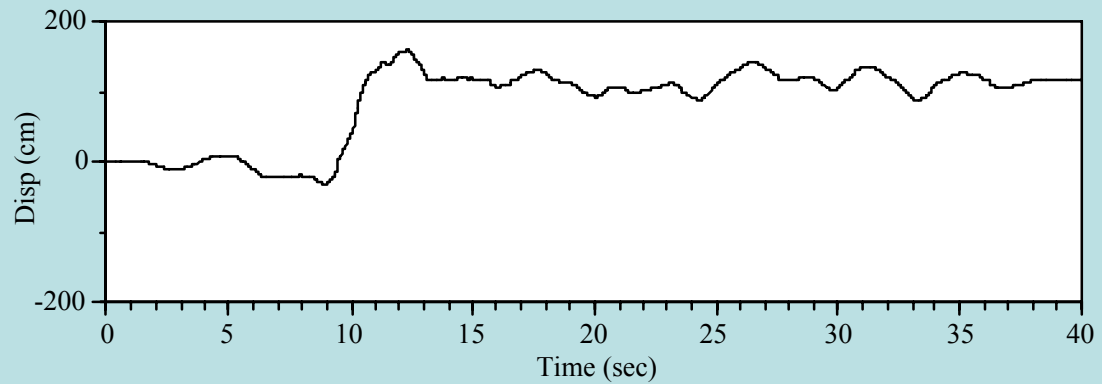
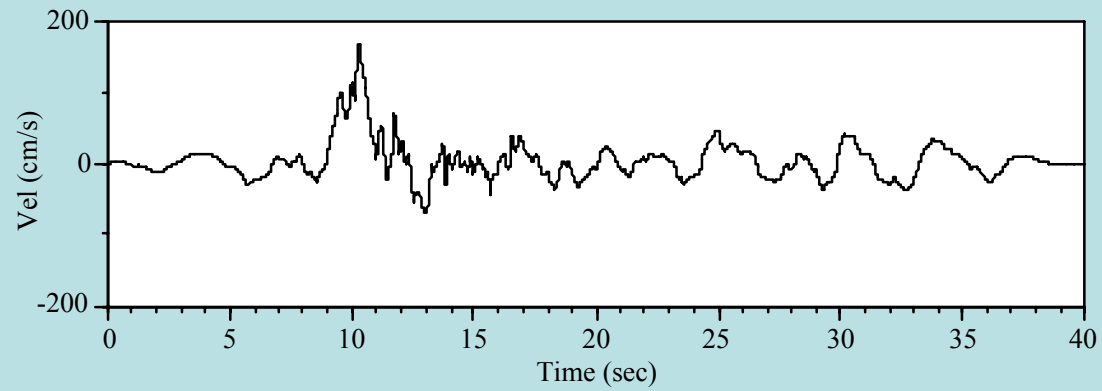
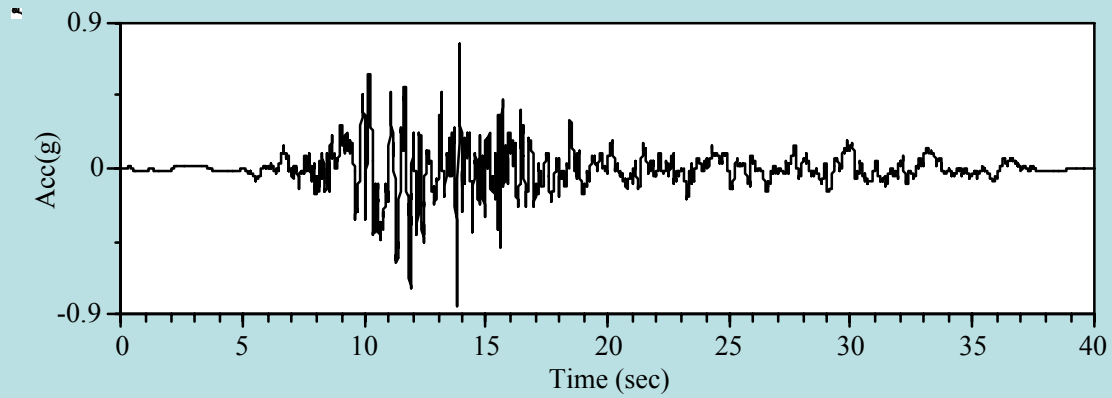
# Issues for Combining Fling and Vibratory Ground Motion

- What is the timing between fling and S-waves?
  - For sites close to the fault, fling arrives near the S-wave
- Polarity of fling and S-waves?
  - For design ground motions, require constructive interference of velocity

# Example Timing of Fling



# Ground Motion with Fling



# Average Spectrum Including Fling

